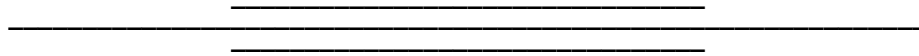


STATE OF GEORGIA STATE ROAD & TOLLWAY AUTHORITY



PROJECT SUMMARY REPORT

Fulton County

Northridge Road Bridge Improvements Over SR 400

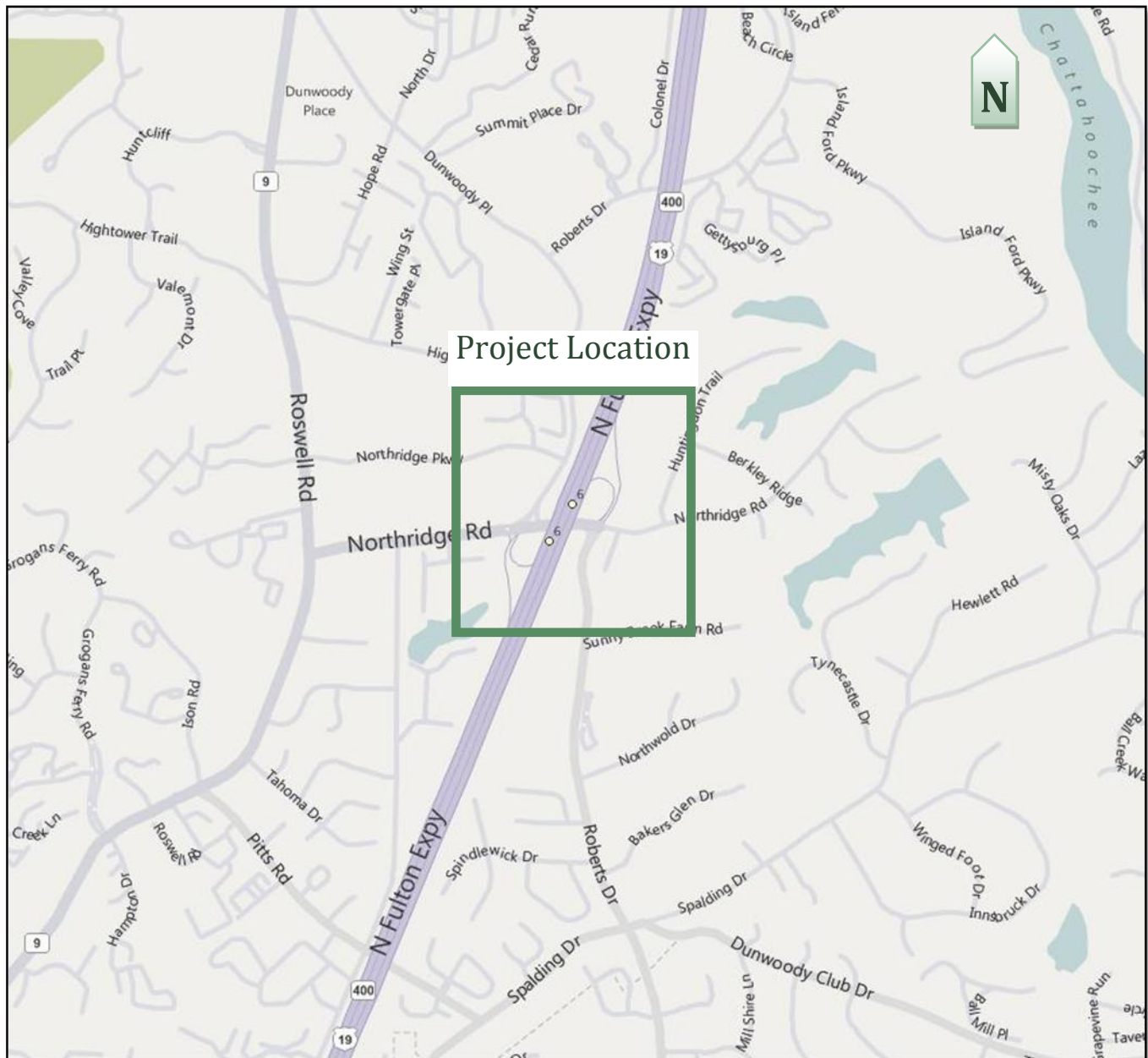
Federal Route No.: US 19
State Route No.: SR 400



City of Sandy Springs

December 2, 2010

Northridge Road Bridge Over SR 400



LOCATION SKETCH | NOT TO SCALE

ROADWAY DESIGN OVERVIEW

Route Number

Georgia 400 (SR 400) at Northridge Road

Location

The project is located in the City of Sandy Springs in Fulton County, Georgia, at the interchange of SR 400 and Northridge Road.

Project Description

The interchange of Northridge Road and SR 400 is located in Fulton County, within the City of Sandy Springs. The interchange is a partial cloverleaf design with loop ramps in the northeastern and southwestern quadrants. Both of the loop ramps utilize a taper-type exit along SR 400.

The Northridge Road Bridge over SR 400, which was built in 1968, currently relays a total of five lanes across the structure. The lane widths vary and are not consistent between lanes. The smallest lane measures approximately 8 feet across. A 5-foot-wide sidewalk runs along the south side of the bridge with no sidewalk on the north side. The existing structure does not have concrete bridge parapets but instead utilizes a system of metal side railings, which are bolted to the bridge.

Signalized intersections exist on both ends of the heavily traveled Northridge Road Bridge. To the west of the bridge, Dunwoody Place intersects Northridge Road and the southbound SR 400 ramps. Beyond this intersection, Northridge Road eventually dead ends into Roswell Road. Dunwoody Place serves as the main arterial connecting the residents of Roswell to SR 400. To the east of the bridge, Roberts Drive intersects Northridge Road and the northbound SR 400 ramps. Beyond this intersection, there are residential streets and neighborhoods.

Very few pedestrian facilities are provided within the area of the bridge. The sidewalk running along the south side of the bridge has limited connectivity to pedestrian facilities to the east of the bridge. No curb ramps are provided in this area to allow pedestrians to cross the intersection of Roberts Drive. There is no pedestrian system beyond the western extents of the bridge structure.

Poor traffic conditions exist across the bridge, especially during peak hours of travel. Factors that contribute to this congestion include narrow lane widths across the bridge, confusion with lane assignments, turn lanes with insufficient storage capacity, and large volumes of vehicles at conflict points. Additionally, accident data, which is provided within the Traffic portion of this document, indicates a high rate of accidents along the bridge.

The taper-type northbound off-ramp exits SR 400 just after the Northridge Road Bridge and immediately transitions into a looping movement. Drivers approaching this exit are not provided with a clear view of the gore, as the existing bridge structure obscures the exit point. The loop ramp transitions from one lane to two lanes as it approaches the intersection with Northridge Road. Because of the skew and the short length of the ramp, drivers are not provided with enough distance and signage to indicate the uses and lane assignments for these two lanes. Moreover, the storage length at the top of the ramp for

the through movement at the intersection is not long enough, causing the traffic waiting to cross to Roberts Drive to back up. During regular operation, the backup attributed to this lane blocks one of the right-turning movements onto Northridge Road and during peak hours the backup extends all the way down the ramp to SR 400.

Traffic congestion at the Northridge Road Bridge could be alleviated and a safer traveling environment could be attained by widening the bridge structure, by modifying the northbound off-ramp, and by introducing improvements at the mainline intersections with Roberts Drive and Dunwoody Place.

Bridge widening is proposed along both the north and south sides of the existing bridge structure in order to improve the inadequate railings. On the south side of the bridge, the deck would be saw cut at the second beam and a current Georgia DOT standard parapet and sidewalk would be constructed. Construction in this area would include the addition of a new sidewalk, a bike lane, and a side barrier. The majority of the bridge widening and all the substructure construction would occur on the north side of the structure. Additional space would be constructed to widen the five existing lanes to seven proposed lanes with full 12-foot widths. The new typical section would also add a dedicated slip ramp movement from the northbound SR 400 ramps to the northbound lanes of Dunwoody Place.

Additionally, this project will determine whether any improvements can be made to the northbound off-ramp and to the signage for lane delineation.

Improvements to the Roberts Drive and Dunwoody Place intersections would relieve traffic congestion. In addition, the slip movement from the SR 400 northbound exit would increase safety and reduce traffic congestion by the elimination of a conflict point across the bridge. Additional turn lane storage would also facilitate efficient movement through the project intersections and across the bridge. These modifications to Roberts Drive and Dunwoody Place would be designed to integrate with other future projects planned for this area.

Other design considerations in this area include coordination of existing utilities running beneath the Northridge Road Bridge, improved pedestrian and bicycling facilities, and the replacement of guardrail where necessary. Staging of the project would be designed to utilize the existing elements of the bridge, but may require temporary lane reductions. Additionally, there are no anticipated permanent right of way impacts. All construction is intended to be accommodated within the existing right of way; however, temporary easements will be required.

Existing Right of Way

SR 400:	370 ft (Corridor) to 800 ft (at Ramps)
Northridge Road:	60 ft to 80 ft
Roberts Drive:	70 ft minimum
Dunwoody Place:	Included within Freeway Right of Way

Estimated Project Length

2,600 feet along Northridge Road

PDP Class

Major Interchange

Functional Class

SR 400:	Urban Freeway / Expressway
Northridge Road:	Minor Urban Arterial
Roberts Drive:	Minor Urban Arterial
Dunwoody Place:	Minor Urban Arterial

Anticipated Design Considerations

- Utility relocations required
- Bridge deck overlay or replacement likely required
- Bridge maintenance required (patching, epoxy injection, etc.)
- Retrofitting of bridge cap cantilevers required
- Transportation Management Plan for FHWA likely required
- GEPA documentation as required
- GDOT letting with State Road and Tollway Authority Funding
- Drilled shaft alternatives possible
- Potential for required traffic analysis of temporary traffic control staging
- Accelerated schedule possible with streamlined Plan Development Process (PDP)
- Restricted hours required
- Required design life of 10 years minimum until replacement by programmed Managed Lane Project along SR 400

Design Speed

SR 400:	70 MPH
Northridge Road:	45 MPH
Roberts Drive:	45 MPH
Dunwoody Place:	45 MPH

Potential Typical Section

SR 400:

- Existing freeway typical to remain

Northridge Road Bridge:

- Standard side barrier, 6' sidewalk, 5' bike lane, 12' lane as a through right, 12' lane as a left turn, 12' lane as a shared left turn, 12' lane as a through, 12' lane as a through right, 5' bike lane, 2' raised slip island, 2' gutter offset, 12' slip lane, 4' shoulder, straight bridge parapet

Roberts Drive:

- Additional right-turn lane and left-turn lane storage to be provided at Northridge Road intersection

Dunwoody Place:

- Additional widening and accommodations to receive slip movement from SR 400 northbound off-ramp

Access Types

Limited Access:

- US 19 / SR 400

By Permit:

- Northridge Road
- Roberts Drive
- Dunwoody Place

Traffic Control During Staging

Staged construction – maintain traffic on existing roadway and ramps while constructing bridge widening. Potential need for temporary lane reductions across Northridge Road during bridge construction. Existing laneage will likely need to be reduced to four lanes total across the bridge during stage 1.

Bridge construction will be performed in stages:

- Stage 1: Construct bridge widening to the north side
- Stage 2: Shift Northridge Road traffic north 20', remove and reconstruct south-side bridge parapet
- Stage 3: Perform existing bridge deck rehabilitation as required in bridge deck condition survey

SR 400: Potential need to temporarily shift the four mainline through lanes toward the northbound exit lane to provide working space for the center bent substructure construction. This will need to be further analyzed as design develops and shall be included in the Transportation Management Plan.

Right of Way Requirements

It is the project intent to contain the construction within the existing right of way with the use of temporary easements where necessary.

BRIDGE DESIGN OVERVIEW

Bridge Summary

The existing bridge is a four-span, concrete deck and steel plate girder on concrete bent structure over SR 400 (US 19). The existing bridge is in fair condition with cracks and spalls on the deck superstructure and substructure.

The proposed structure is a widened and rehabilitated bridge designed to accommodate the new lanes on Northridge Road as well as increasing the width of the existing lanes. The proposed structure will be widened in-kind with no changes to the existing span configuration.

Existing Major Structures

The existing bridge is a four-span, concrete deck and steel plate girder on concrete bent structure over SR 400 (US 19). The bridge is 289 feet long and 61 feet wide with a sidewalk on the south side of the bridge. The roadway is 52 feet wide and carries five lanes of traffic with varying lane widths. The existing railings are steel post and rail bolted to the bridge deck. The existing substructure consists of concrete bents on spread footings with one abutment pile supported and the other supported on spread footings.

Existing Bridge Condition

The existing bridge is in fair condition. The sufficiency rating of the structure is 54.5 percent. The bridge railings do not meet the current minimum requirements for safety. There are cracks and spalls on the substructure and deck superstructure that require maintenance. The most recent bridge inventory report gives good ratings to the deck, superstructure, and substructure, but rates the overall condition of the bridge as fair.

Proposed Major Structures

The proposed structure is a widened and rehabilitated bridge designed to accommodate the new lanes on Northridge Road as well as increasing the width of the existing lanes. The structure will also be designed with a new wider sidewalk on the south side of the bridge, bike lanes on both sides of the bridge, and new parapets on both sides of the bridge that meet minimum safety requirements. The existing alignment will be shifted 4 feet to the north to accommodate the wider sidewalk and bike lane on the south side of the bridge. This alignment shift will limit the additional substructure construction to the north side of the bridge only. Replacement of the sidewalk and parapet will widen the south side of the bridge deck, but will not require new girders or substructure. There are no vertical clearance issues with widening to the north side of the bridge.

The proposed structure widening will be in-kind, with no changes to the span configuration. The portion of the existing bridge to remain will be rehabilitated according to the recommendations of the recent bridge condition and deck condition surveys provided by the Georgia DOT Bridge Maintenance Office. The most likely deck rehabilitation options are a concrete overlay or a complete deck replacement. Neither survey was available at the time of this report.

The bridge will be widened approximately 53 feet, 7½ inches to the north of the existing alignment to accommodate the additional travel lanes. The southern side of the bridge will be reconfigured to add the 6-foot sidewalk and 4-foot bike lane. The total proposed bridge width is 110 feet, 10 inches. The widening will consist of a cast-in-place concrete deck on steel plate girder superstructure with concrete bents on spread footings and a pile-supported abutment substructure to match the existing bridge. The foundation system for the concrete bents may require drilled shafts instead of driven piles or spread footings in order to limit the disruption to traffic on SR 400 during construction of the bridge. A new Bridge Foundation Investigation (BFI) will be needed to determine the foundation type required for the widened portion of the bridge. The existing substructure will be analyzed for the loads of the new widened bridge. The existing substructure cantilever may need to be reinforced to support the loads of the new widened bridge.

TRAFFIC DESIGN OVERVIEW

Traffic – Current ADT

Northridge Road – 2009 AADT – 38,340

Existing Traffic Control

Existing traffic control consists of traffic signals at SR 400 northbound ramps/Roberts Drive at Northridge Road, and SR 400 southbound ramps/Dunwoody Place at Northridge Road. The signal at SR 400 northbound ramp/Roberts Drive at Northridge Road currently operates split phased on the mainline of Northridge Road/Northridge Drive.

Existing Traffic Conditions

A review of traffic data and field observations shows that the Northridge Road at SR 400 interchange has substantial operational issues. The predominant movement for SR 400 northbound traffic exiting at Northridge Road is vehicles traveling to Dunwoody Place. Currently only one right-turn lane exists for traffic traveling west on Northridge Road to Dunwoody Place. There is an 85/15 traffic split between Dunwoody Place and Northridge Road west for SR 400 northbound traffic in both the a.m. and p.m. peak hours. Westbound Northridge Road experiences significant weaving conflicts between vehicles turning left onto Northridge Road from Roberts Drive and vehicles exiting SR 400 North going to Dunwoody Place. The weaving impacts the off-ramp from SR 400, with traffic spilling back to the off-ramp from SR 400 in the p.m. peak period.

The eastbound lanes of Northridge Road at the SR 400 northbound ramp consist of a dedicated left-turn lane and a shared through left lane. The left-turn lane is short and not capable of handling the demand of left-turning traffic onto SR 400 North. Fifty percent of the vehicles traveling east on Northridge Road turn left onto SR 400 North. The traffic queues back and blocks the left-turn lane onto the SR 400 southbound ramp.

A full traffic analysis will be performed during the design phase of the project. The a.m. and p.m. peak periods will be studied, and the intersections of SR 400 at Roberts Drive and SR 400 at Dunwoody Place will be analyzed using the Highway Capacity methodology and Synchro.

Accident Data – Northridge Road

Northridge and Roberts Accident History Last 3 Years							
2009 Accident History (34 accidents up to 12-11-09)							
Northbound	# of accidents	Southbound	# of accidents	Eastbound	# of accidents	Westbound	# of accidents
Rear End	8	Rear End	5	Angle	3	Rear End	1
Angle	3	Side Swipe	2	Side Swipe	3		
Side Swipe	2	Angle	4	Rear End	3		
Number of accidents per direction	13		11		9		1
2008 Accident History (43 accidents)							
Northbound	# of accidents	Southbound	# of accidents	Eastbound	# of accidents	Westbound	# of accidents
Rear End	4	Rear End	8	Angle	3	Rear End	3
Side Swipe	3	Side Swipe	1	Side Swipe	1	Angle	1
Ran Off Road	1	Angle	3	Rear End	12	Side Swipe	1
		Head to Head	1	Ran Off Road	1		
Number of accidents per direction	8		13		17		5
2007 Accident History (31 accidents)							
Northbound	# of accidents	Southbound	# of accidents	Eastbound	# of accidents	Westbound	# of accidents
Rear End	2	Rear End	8	Angle	1	Rear End	6
Side Swipe	3	Side Swipe	2	Side Swipe	2	Side Swipe	1
		Angle	1	Rear End	5		
Number of accidents per direction	5		11		8		7

Accident data from 2007 to 2009 was analyzed for Northridge Road and Roberts Drive. Rear-end crashes are the most common crashes in the project area, accounting for 60 percent of the total crashes. A possible reason for this is insufficient left-turn storage lanes on the Northridge Road Bridge. Sideswipe and angle crashes account for 36 percent of the total crashes, most likely caused from the weaving movements trying to turn right onto Dunwoody Place. The existing lanes on the Northridge Road Bridge average 9.78 feet per lane, which combined with the weaving movements may also contribute to the sideswipe crashes since drivers have less time to correct from errant maneuvers.

Bicycle and Pedestrian Considerations

There are no destinations on the north side of Northridge Road at SR 400, and prohibiting pedestrians on the north side of the bridge will eliminate conflicts with the free-flow lane from the SR 400 northbound off-ramp to Dunwoody Place. Pedestrians will be allowed to access all four corners of both intersections from new crosswalks and wheelchair ramps, but will not be allowed to cross on the north side of the bridge. Bicycle accommodations will be provided on the bridge only.

PROJECT OVERVIEW

Previous Related Projects

- APD-056-1(10) – North Fulton Expressway - 1967
- HES-056-1(48) – Improvements for Northridge Road & SR 400 – 1988

Other Programmed Projects in the Vicinity

- SR 400 FM I-285 TO MCFARLAND ROAD/FORSYTH CO HOV LANES
- SR 400 ATMS RAMP METERS FM I-285 TO SR 120/OLD MILTON PKWY
- SR 400 FROM CR 209/SPALDING DR TO CR 458/MCFARLAND ROAD
- NON-INTERSTATE LIMITED ACCESS HIGHWAY SIGN UPGRADE - SR 400
- SR 400 FROM I-285 TO WINDWARD PARKWAY

Utilities

Coordination will be done with utility companies during design development. Known and anticipated utilities in the corridor include:

Fulton County Water

Fulton County Sewer

Atlanta Gas Light

AT&T

Georgia Power Distribution

Comcast

Verizon Communications

Georgia DOT ATMS

City of Sandy Springs Interconnect

Schedule

Design

Concept	4 Months
Preliminary Plans	6 Months
Environmental (GEPA)	8 Months (Concurrent with Concept and Preliminary)
Right of Way Plans	2 Months
Right of Way Acquisition	6 Months
Final Plans	6 Months (Concurrent with Right of Way Acquisition)
Contracts	4 Months
Estimated Letting Date	22 Months from NTP
Construction	16 Months
Project Complete	38 – 40 Months from NTP

Estimated Cost

Item	Subtotal Cost	Cost
Right of Way		\$100,000.00
Reimbursable Utilities		\$100,000.00
Construction	\$6,050,837.68	
E&I (6%)	\$363,050.26	
Total Construction		\$6,413,887.94
Design		\$650,000.00
Total Project Cost		\$7,263,887.94

Permits Required

Signal Modification Permit via GDOT
Federal Oversight

Anticipated Level of Environmental Analysis

Compliance with the Georgia Environmental Protection Act (GEPA)

Level of Public Involvement

Public Involvement Open House

Time-Saving Procedures Appropriate

Yes (☒) No (☐)

Design Exceptions Required

None Anticipated

Alternates Considered

No-Build

Attachments

1. Project Sketch (as Roll Plot)
2. Potential Typical Sections
3. Cost Estimate
4. Project Pictures
5. Existing Plans
6. Existing BFI
7. Bridge Inventory Data Listing

ATTACHMENT 2

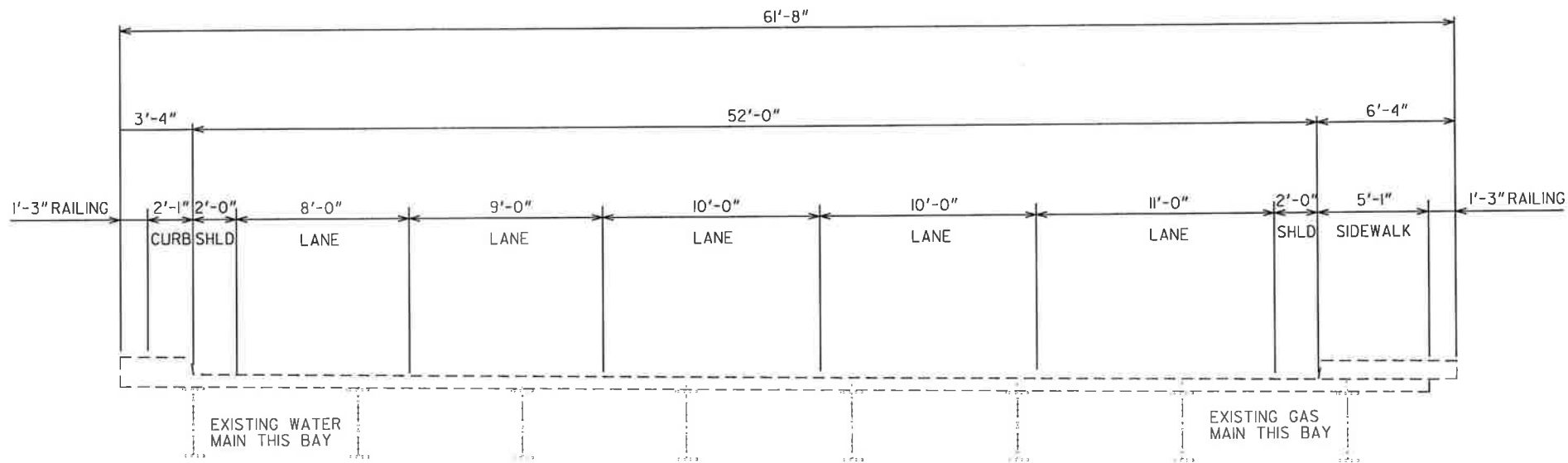
POTENTIAL TYPICAL SECTIONS



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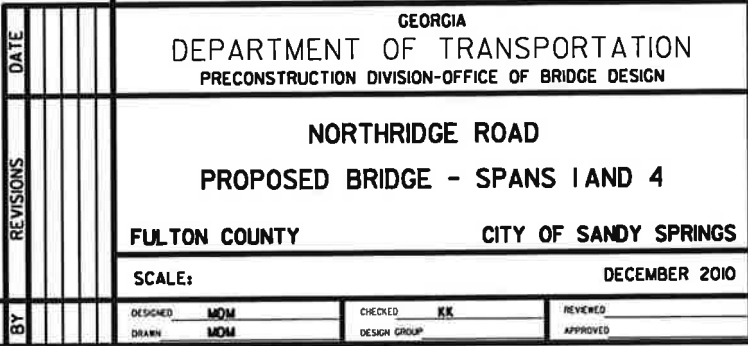
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STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
GA.			



BRIDGE SHEET
1 OF 3

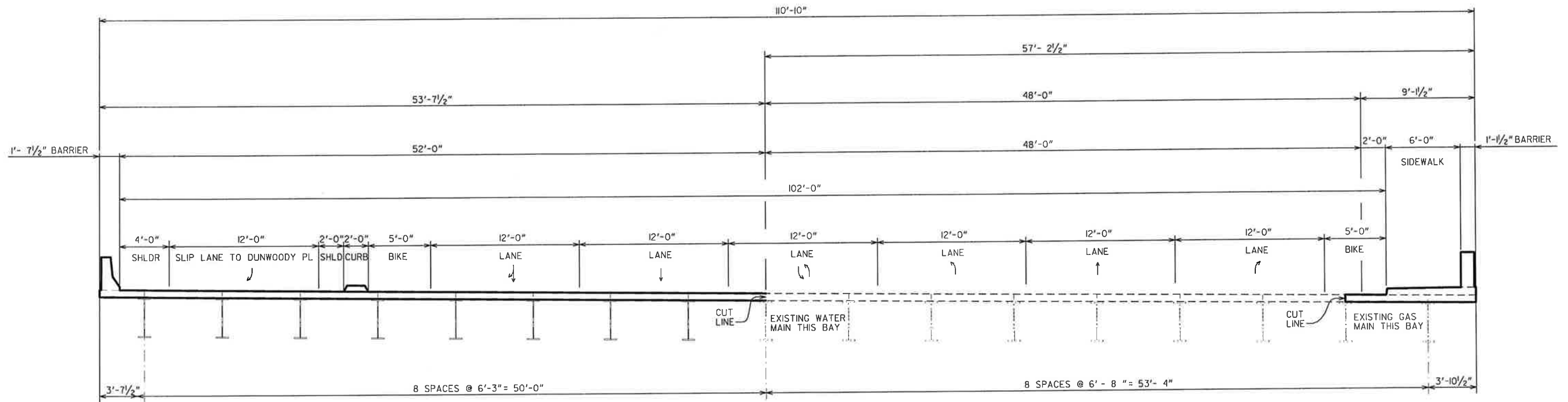
BY	REVISIONS	DATE	GEORGIA DEPARTMENT OF TRANSPORTATION PRECONSTRUCTION DIVISION-OFFICE OF BRIDGE DESIGN	
			NORTHBRIDGE ROAD EXISTING BRIDGE	
			FULTON COUNTY	CITY OF SANDY SPRINGS
			SCALE:	DECEMBER 2010
DESIGNED	MOM	CHECKED	KK	REVIEWED
DRAWN	MOM	DESIGN GROUP		APPROVED



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STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
GA.			



REVISIONS	DATE	
	BY	
	DATE	
	BY	
GEORGIA DEPARTMENT OF TRANSPORTATION PRECONSTRUCTION DIVISION-OFFICE OF BRIDGE DESIGN		
NORTHBRIDGE ROAD PROPOSED BRIDGE - SPANS 2 AND 3		
FULTON COUNTY CITY OF SANDY SPRINGS		
SCALE: DECEMBER 2010		
DESIGNED	MDM	
CHECKED	KK	
DRAWN	MDM	
DESIGN GROUP		
REVIEWED		
APPROVED		

BRIDGE SHEET
3 OF 3

ATTACHMENT 3

COST ESTIMATE



CONSTRUCTION COST ESTIMATE

Project: Northridge Road Bridge Improvements over SR 400

County: FULTON COUNTY

Prepared by:
Last Modified:

 **ARCADIS**
December 2, 2010

ITEM NO.	ITEM	UNIT	QUANTITY	UNIT PRICE	COST
	ROADWAY ITEMS				
150-1000	TRAFFIC CONTROL (PROJECT NO. CSMSL-0009-00(159) & CSMSL-0009-00(160))	LS	LUMP	\$700,000.00	\$700,000.00
150-5010	TRAFFIC CONTROL, PORTABLE IMPACT ATTENUATOR	EA	4	\$5,115.84	\$20,463.36
207-0203	FOUND BK FILL MATL, TP II	CY	50	\$37.36	\$1,868.00
210-0100	GRADING COMPLETE (PROJECT NO. CSMSL-0009-00(159) & CSMSL-0009-00(160))	LS	LUMP	\$550,000.00	\$550,000.00
310-1101	GR AGGR BASE CRS, INCL MATL	TN	8400	\$14.96	\$125,664.00
318-3000	AGGR SURF CRS	TN	50	\$17.12	\$856.00
402-1812	RECYCLED ASPH CONC LEVELING, INCL BITUM MATL & H LIME	TN	500	\$75.00	\$37,500.00
402-3121	REC ASPH CONC 25mm SUPERPAVE, GP 1 OR 2, INCLUDE BIT MATL & H LIME	TN	3420	\$65.00	\$222,300.00
402-3130	RECYCLED ASPH CONC 12.5 MM SUPERPAVE, GP 2 ONLY, INCL BITUM MATL & H LIME	TN	330	\$70.00	\$23,100.00
402-3190	REC ASPH CONC 19mm SUPERPAVE, GP 1 OR 2, INCLUDE BIT MATL & H LIME	TN	855	\$70.00	\$59,850.00
402-4510	RECYCLED ASPH CONC 12.5 MM SUPERPAVE, GP 2 ONLY, INCL POLYMER-MODIFIED BIT	TN	2025	\$75.00	\$151,875.00
413-1000	BITUM TACK COAT	GL	4000	\$1.72	\$6,880.00
431-1000	GRIND CONC PVT	SY	5000	\$2.40	\$12,000.00
432-0206	MILL ASPH CONC PVT, 1 1/2 IN DEPTH	SY	20800	\$1.25	\$26,000.00
430-0200	PLAIN PC CONC PVT, CL 1 CONC, 10 INCH THK	SY	1500	\$35.00	\$52,500.00
433-1000	REINF CONC APPROACH SLAB	SY	350	\$136.24	\$47,684.00
441-0104	CONC SIDEWALK, 4 IN	SY	620	\$23.65	\$14,663.00
441-0204	PLAIN CONC DITCH PAVING, 4 IN	SY	500	\$24.70	\$12,350.00
441-0748	CONCRETE MEDIAN, 6 IN	SY	1160	\$29.04	\$33,686.40
441-6022	CONC CURB & GUTTER, 6 IN X 30 IN, TP 2	LF	4000	\$11.58	\$46,320.00
452-1000	FULL DEPTH SLAB REPLACEMENT	CY	60	\$254.56	\$15,273.60
446-1100	PVT REINF FABRIC STRIPS, TP 2, 18 INCH WIDTH	LF	2820	\$3.12	\$8,798.40
461-1000	RESEALING ROADWAY JOINTS AND CRACKS	LF	1200	\$1.40	\$1,680.00
500-9999	CLASS B CONC, BASE OR PVT WIDENING	CY	100	\$163.61	\$16,361.00
550-1120	STORM DRAIN PIPE, 12 IN, H 1-10	LF	110	\$46.60	\$5,126.00
550-1150	STORM DRAIN PIPE, 15 IN, H 1-10	LF	70	\$21.47	\$1,502.90
550-1180	STORM DRAIN PIPE, 18 IN, H 1-10	LF	800	\$29.26	\$23,408.00
550-4218	FLARED END SECTION 18 IN, STORM DRAIN	EA	4	\$451.98	\$1,807.92
576-1018	SLOPE DRAIN PIPE, 18 IN	LF	120	\$29.11	\$3,493.20
609-1000	REMOVE ROADWAY SLAB	SY	216	\$31.37	\$6,775.92
611-3010	RECONSTR DROP INLET, GROUP 1	EA	6	\$1,263.06	\$7,578.36
611-3030	RECONSTR STORM SEW MANHOLE, TYPE 1	EA	4	\$1,281.70	\$5,126.80
620-0100	TEMPORARY BARRIER, METHOD NO. 1	LF	1100	\$24.48	\$26,928.00
621-3150	CONCRETE BARRIER, TYPE 26	LF	125	\$300.00	\$37,500.00
621-4080	CONCRETE SIDE BARRIER, TYPE 7R	LF	230		
632-0003	CHANGEABLE MESSAGE SIGN, PORTABLE, TYPE 3	EA	4	\$6,864.58	\$27,458.32

641-1100	GUARDRAIL, TP T	LF	342	\$42.45	\$14,517.90
641-1200	GUARDRAIL, TP W	LF	1520	\$14.56	\$22,131.20
641-5001	GUARDRAIL ANCHORAGE, TP 1	EA	10	\$636.40	\$6,364.00
641-5012	GUARDRAIL ANCHORAGE, TP 12	EA	5	\$2,275.34	\$11,376.70
668-1100	CATCH BASIN, GP 1	EA	14	\$2,112.83	\$29,579.62
668-4300	STORM SEWER MANHOLE, TP 1	EA	4	\$1,958.32	\$7,833.28
	EROSION CONTROL				
163-0232	TEMPORARY GRASSING	AC	2	\$296.36	\$592.72
163-0240	MULCH	TN	70	\$144.95	\$10,146.50
163-0300	CONSTRUCTION EXIT	EA	6	\$922.26	\$5,533.56
163-0550	CONSTRUCT AND REMOVE INLET SEDIMENT TRAP	EA	50	\$142.47	\$7,123.50
165-0030	MAINTENANCE OF TEMPORARY SILT FENCE, TP C	LF	12000	\$0.68	\$8,160.00
165-0101	MAINTENANCE OF CONSTRUCTION EXIT	EA	6	\$441.53	\$2,649.18
165-0105	MAINTENANCE OF INLET SEDIMENT TRAP	EA	52	\$53.08	\$2,760.16
167-1000	WATER QUALITY MONITORING AND SAMPLING	EA	2	\$412.56	\$825.12
167-1500	WATER QUALITY INSPECTIONS	MO	18	\$511.37	\$9,204.66
171-0030	TEMPORARY SILT FENCE, TYPE C	LF	12000	\$2.84	\$34,080.00
700-6910	PERMANENT GRASSING	AC	4	\$669.77	\$2,679.08
700-7000	AGRICULTURAL LIME	TN	8	\$52.05	\$416.40
700-7010	LIQUID LIME	GL	10	\$16.03	\$160.30
700-8000	FERTILIZER MIXED GRADE	TN	4	\$400.18	\$1,600.72
700-8100	FERTILIZER NITROGEN CONTENT	LB	180	\$2.24	\$403.20
716-2000	EROSION CONTROL MATS, SLOPES	SY	2600	\$0.94	\$2,444.00
	SIGNING AND MARKING / SIGNAL ITEMS				
615-1200	DIRECTIONAL BORE - 3 IN	LF	55	\$8.40	\$462.00
615-1200	DIRECTIONAL BORE - 5 IN	LF	750	\$10.00	\$7,500.00
615-1200	DIRECTIONAL BORE - 6 IN	LF	540	\$12.00	\$6,480.00
636-1020	HIGHWAY SIGNS, TP 1 MATL, REFL SHEETING, TP 3	SF	135	\$13.34	\$1,800.90
636-1033	HIGHWAY SIGNS, TP 1 MATL, REFL SHEETING, TP 9	SF	77	\$17.88	\$1,376.76
636-1041	HIGHWAY SIGNS, TP 2 MATL, REFL SHEETING, TP 9	SF	137	\$27.60	\$3,781.20
636-2070	GALV STEEL POSTS, TP 7	LF	511	\$6.77	\$3,459.47
639-3004	STEEL STRAIN POLE, TP IV, INCL LUMINAIRE ARM W/ 55 FT MAST ARM	LF	2	\$10,000.00	\$20,000.00
639-3004	STEEL STRAIN POLE, TP IV, INCL LUMINAIRE ARM W/ 65 FT MAST ARM	EA	8	\$12,500.00	\$100,000.00
639-4004	STRAIN POLE, TP IV	EA	4	\$5,363.84	\$21,455.36
647-1000	TRAFFIC SIGNAL INSTALLATION NO. 1	LS	LS	\$82,000.00	\$82,000.00
647-1000	TRAFFIC SIGNAL INSTALLATION NO. 2	LS	LS	\$82,000.00	\$82,000.00
647-2160	PULL BOX, PB-6	EA	4	\$954.05	\$3,816.20
647-2170	PULL BOX, PB-7	EA	2	\$1,437.54	\$2,875.08
653-0110	THERMOPLASTIC PVMT MARKING, ARROW, TP 1	EA	6	\$70.67	\$424.02
653-0120	THERMOPLASTIC PVMT MARKING, ARROW, TP 2	EA	16	\$68.37	\$1,093.92
653-0210	THERMOPLASTIC PVMT MARKING, WORD, TP 1	EA	3	\$108.98	\$326.94
653-1501	THERMOPLASTIC SOLID TRAF STRIPE, 5 IN, WHITE	LF	6240	\$0.32	\$1,996.80
653-1502	THERMOPLASTIC SOLID TRAF STRIPE, 5 IN, YELLOW	LF	3840	\$0.33	\$1,267.20
653-1704	THERMOPLASTIC SOLID TRAF STRIPE, 24 IN, WHITE	LF	252	\$3.61	\$909.72
653-1804	THERMOPLASTIC SOLID TRAF STRIPE, 8 IN, WHITE	LF	2230	\$1.71	\$3,813.30
653-1810	THERMOPLASTIC SOLID TRAF STRIPE, 10 IN, WHITE	LF	835	\$0.84	\$701.40
653-3501	THERMOPLASTIC SKIP TRAF STRIPE, 5 IN, WHITE	GLF	4000	\$0.22	\$880.00
653-3502	THERMOPLASTIC SKIP TRAF STRIPE, 5 IN, YELLOW	GLF	1600	\$2.57	\$4,112.00
653-6004	THERMOPLASTIC TRAF STRIPING, WHITE	SY	1015	\$2.51	\$2,547.65
653-6006	THERMOPLASTIC TRAF STRIPING, YELLOW	SY	450	\$2.57	\$1,156.50
654-1001	RAISED PVMT MARKERS TP 1	EA	60	\$2.93	\$175.80
654-1003	RAISED PVMT MARKERS TP 3	EA	270	\$3.29	\$888.30
657-1054	PREFORMED PLASTIC SOLID PVMT MKG, 5 IN, WHITE, TP PB	LF	2145	\$4.30	\$9,223.50
657-1244	PREFORMED PLASTIC SOLID PVMT MKG, 24 IN, WHITE, TP PB	LF	24	\$4.30	\$103.20
657-3054	PREFORMED PLASTIC SKIP PVMT MKG, 5 IN, WHITE, TP PB	GLF	845	\$4.50	\$3,802.50

657-5002	PREFORMED PLASTIC PAVEMENT MARKING, YELLOW, TP PB	SY	570	\$6.00	\$3,420.00
657-5017	PREFORMED PLASTIC PVMT MKG, WORDS AND/OR SYM, ARROW, TP 2, WHITE, TP PB	EA	6	\$480.83	\$2,884.98
657-6054	PREFORMED PLASTIC SOLID PVMT MKG, 5 IN, YELLOW, TP PB	LF	965	\$3.92	\$3,782.80
682-6120	CONDUIT, RIGID, 2 IN	LF	60	\$8.00	\$480.00
682-6222	CONDUIT, NONMETAL, TP 2, 2 IN	LF	100	\$5.00	\$500.00
682-6233	CONDUIT, NONMETL, TP 3, 2 IN	LF	5200	\$3.05	\$15,860.00
682-6236	CONDUIT, NONMETAL, TP 2 - POWER SERVICE, 2 IN	LF	630	\$6.00	\$3,780.00
935-1118	OUTSIDE PLANT FIBER OPTIC CABLE, LOOSE TUBE, SINGLE MODE, 144 FIBER	LF	4200	\$1.61	\$6,762.00
935-1511	OUTSIDE PLANT FIBER OPTIC CABLE, DROP SINGLE MODE, 6 FIBER	LF	150	\$1.80	\$270.00
935-3101	FIBER OPTIC CLOSURE, UNDERGROUND, 6 FIBER	EA	2	\$450.00	\$900.00
935-3102	FIBER OPTIC CLOSURE, UNDERGROUND, 12 FIBER	EA	2	\$656.25	\$1,312.50
935-3108	FIBER OPTIC CLOSURE, UNDERGROUND, 144 FIBER	EA	2	\$1,200.00	\$2,400.00
935-3602	FIBER OPTIC CLOSURE, FDC PRE-TERMINATED, TYPE A, 6-FIBER	EA	2	\$219.11	\$438.22
935-4010	FIBER OPTIC SPLICE, FUSION	EA	200	\$24.42	\$4,884.00
935-8000	TESTING (COMMUNICATION AND ELECTRONIC EQUIPMENT)	LS	LS	\$1,500.00	\$1,500.00
937-6050	INTERSECTION VIDEO DETECTION SYSTEM ASSEMBLY, TYPE A	EA	4	\$3,913.85	\$15,655.40
937-6051	INTERSECTION VIDEO DETECTION SYSTEM ASSEMBLY, TYPE B	EA	2	\$4,600.00	\$9,200.00
937-6150	PROGRAMMING MONITOR, TYPE A	EA	1	\$381.74	\$381.74
938-8000	TESTING (IVDS)	LS	1	\$2,000.00	\$2,000.00
939-2305	FIELD SWITCH, TYPE C	EA	2	\$1,892.17	\$3,784.34
	BRIDGE ITEMS				
441-0004	CONC SLOPE PAV, 4 IN	SY	700	\$38.67	\$27,069.00
500-1000	BRIDGE WIDENING - BRIDGE NO. 1 (NORTHRIDGE ROAD OVER SR 400 - STEEL SUBSTR)	SF	18980	\$140.00	\$2,657,200.00
519-0520	BRIDGE DECK REPLACEMENT (292' X 46)	SY	1492	\$120.00	\$179,040.00
521-3000	PATCHING CONCRETE BRIDGE	SF	40	\$204.00	\$8,160.00
528-0501	EPOXY PRESSURE INJECTION OF CONCRETE CRACKS	LF	50	\$246.00	\$12,300.00
540-1202	REMOVAL OF PARTS OF EXISTING BRIDGE, BR NO - 1	LS	LUMP	\$180,000.00	\$180,000.00
620-0200	TEMPORARY BARRIER, METHOD NO 2	LF	300	\$26.27	\$7,881.00
643-1152	CH LK FENCE, ZC COAT, 6 FT, 9 GA	LF	600	\$22.67	\$13,602.00

Subtotal Construction Cost	\$ 6,050,837.68
Engineering & Inspection (6%)	\$ 363,050.26
Total Construction Cost	\$ 6,413,887.94
Reimbursable Utilities	\$ 100,000.00
Right-of-way	\$ 100,000.00
Project Design	\$ 650,000.00
Grand Total Project Cost	\$ 7,263,887.94

ATTACHMENT 4

PROJECT PICTURES





Existing bridge looking north.



Existing bridge looking north (2).



Existing gas line utility hung beneath bridge.



Existing gas line utility hung beneath bridge (2).



Existing waterline and communications duct bank on northern side of bridge.



Existing bridge, looking west.



Existing intermediate bridge cap showing the utilization of pedestals for short span beams.



Existing view beneath bridge looking across SR 400 and showing median bent / barrier construction.



Existing pipe rail parapet system and sidewalk on south side of bridge. This will need to be upgraded to current Georgia DOT standards.



Existing pipe rail parapet on the north side of the bridge, showing construction date of 1968. This parapet will be replaced and upgraded with the widening on the north side of the bridge.



Intersection of Roberts Drive / SR 400 Northbound Off-Ramp at Northridge Road, looking east.



Intersection of Roberts Drive / SR 400 Northbound Off-Ramp at Northridge Road, looking north.



Looking east toward residential neighborhoods, east of the project.



Intersection of Roberts Drive / SR 400 Northbound Off-Ramp at Northridge Road, looking north.



Existing bridge deck, showing some previous crack repair.

ATTACHMENT 5

EXISTING PLANS



RAMP N-3 CURVE DATA

CURVE NO. 1	CURVE 2
P.I. Sta. = 6+44.81	P.I. Sta. = 13+03.88
Δ = 44° 00'	Δ = 31° 02' 10"
D = 15° 40'	D = 3° 00'
R = 866.00'	R = 1145.92'
T = 147.87'	T = 318.18'
L = 281.07'	L = 620.72'
S.E. = 0.06'/ft.	S.E. = 0.04'/ft.

RAMP N-4 CURVE DATA

CURVE NO. 3	CURVE NO. 2	CURVE NO. 1
P.I. Sta. = 16+52.16	P.I. Sta. = 17+55.82	P.I. Sta. = 10+25.84
Δ = 45° 31' 09"	Δ = 153° 49' 05"	Δ = 36° 00'
D = 24° 55'	D = 88° 12'	D = 24° 55'
R = 230.00'	R = 150.00'	R = 280.00'
T = 96.49'	T = 700.20'	T = 74.73'
L = 182.73'	L = 407.94'	L = 144.51'
S.E. = 0.05'/ft.	S.E. = 0.08'/ft.	S.E. = 0.08'/ft.

ROBERTS DR. CURVE #2

P.I. Sta. = 20+13.85
Δ = 42° 08'
D = 8° 00'
R = 716.20'
T = 275.88'
L = 526.63'
S.E. = 0.04'/ft.

CURVE DATA

P.I. Sta. = 45° 30'
Δ = 16° 37'
D = 350'
T = 161.35'
L = 302.38'

NORTHIDGE ROAD CURVE DATA

P.I. Sta. = 11+62.59
Δ = 17° 03' 29"
D = 3° 00'
R = 1309.86'
T = 256.42'
L = 565.60'
1/8" / FT. X-SLOPE

EQUALITY:
STA. 6+44+04.20 B.K. =
STA. 5+40.14 AHD.

ROBERTS DRIVE CURVE NO. 1

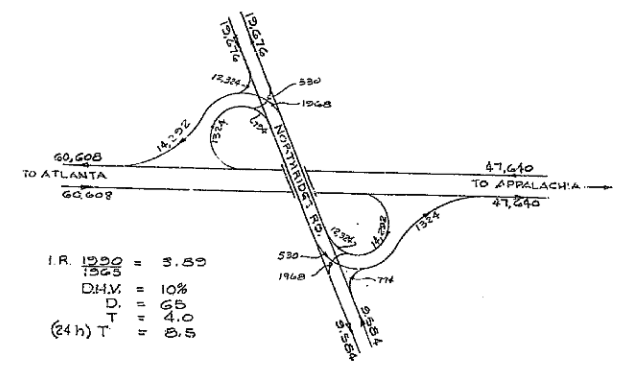
P.I. Sta. = 6+23.69
Δ = 48° 08' 30"
D = 11° 45' 32"
R = 500'
T = 223.36'
L = 420.12'
S.E. = 0.06'/ft.

RAMP N-1 CURVE DATA

CURVE NO. 1	CURVE NO. 2	CURVE NO. 3
P.I. Sta. = 9+82.60	P.I. Sta. = 16+16.91	P.I. Sta. = 17+13.00
Δ = 41° 30'	Δ = 148° 22' 34"	Δ = 61° 06' 01"
D = 24° 55'	D = 38° 12'	D = 24° 55'
R = 230.00'	R = 150.00'	R = 280.00'
T = 87.14'	T = 54.78'	T = 133.75'
L = 166.59'	L = 291.07'	L = 245.27'
S.E. = 0.05'/ft.	S.E. = 0.08'/ft.	S.E. = 0.05'/ft.

RAMP N-2 CURVE DATA

CURVE NO. 1	CURVE NO. 2
P.I. Sta. = 6+85.90	P.I. Sta. = 13+58.56
Δ = 42° 30'	Δ = 30° 00'
D = 15° 40'	D = 5° 00'
R = 366.00'	R = 1145.92'
T = 146.02'	T = 307.05'
L = 277.87'	L = 600.00'
S.E. = 0.06'/ft.	S.E. = 0.04'/ft.



STATE AND PROJECT NO.	REGION NO.	STATE	FEDERAL AID PROJECT NO.	FEDERAL AID FISCAL YEAR	PROJECT NO.	TOTAL SHEETS
3	GA.	APD-1366/44	67	197	218	

REFERENCE DRAWINGS

Drawings listed below show general details for all bents and abutments included in contract. Also set of drawings:

- "Miscellaneous Superstructure Details"
- "Miscellaneous Substructure Details"
- "Bearing Details"
- "Miscellaneous General Details"

SUMMARY OF QUANTITIES

705	Excavate Bridge Excavation
247	Set Yds. Concrete Slope Paving, 4 inch
1000	Superstructure Concrete, Bridge No. 13
365.0	Set Yds. Grade & Concrete
1000	Structure Steel, Bridge No. 13
1000	Structure Steel, Reinforcement Steel, Bridge No. 13
53,676	Set Yds. Reinforcement Steel
558.17	Set Yds. Reinforcement Steel, Bridge No. 13
558.17	Set Yds. Reinforcement Steel, Bridge No. 13
136	Set Yds. Piling, Bridge No. 13
136	Set Yds. Piling, Bridge No. 13
25	Set Yds. Piling, Bridge No. 13
1	Set Yds. Piling, Bridge No. 13
14	Set Yds. Piling, Bridge No. 13

GENERAL NOTES

CONCRETE - All concrete shall be Class A, unless otherwise noted. Chamfer all exposed edges 1/4" unless noted. All chamfers shall be formed with mill cut and dressed chamfer strips.

STRUCTURAL STEEL - Structural steel shall be of the type designated in the plans.

END BENT EXCAVATION - The cost of any excavation necessary for end bent construction shall be included in the over-all bid submitted, except as otherwise provided in Article 105.04. A. of the Standard Specifications.

FINISH - For those areas requiring a Type III - Rubbed Finish, see Page 452 of the Standard Specifications.

PAINT - Structural steel areas which are required to be painted shall be painted with the System III paints, as shown on pages 722-723 of the Standard Specifications.

OTHER REQUIREMENTS - For other General Notes and Contract Requirements, see the Standard Plans (No. 3900, No. 3628 or No. 3631 and No. 9037 modified), and Special Provisions.

WELDING - All welding shall conform to the current Specifications for Welded Highway and Railway Bridges of the American Welding Society.

CLEARING AND GRUBBING - Clearing and grubbing of bridge site shall be paid for as a roadway item.

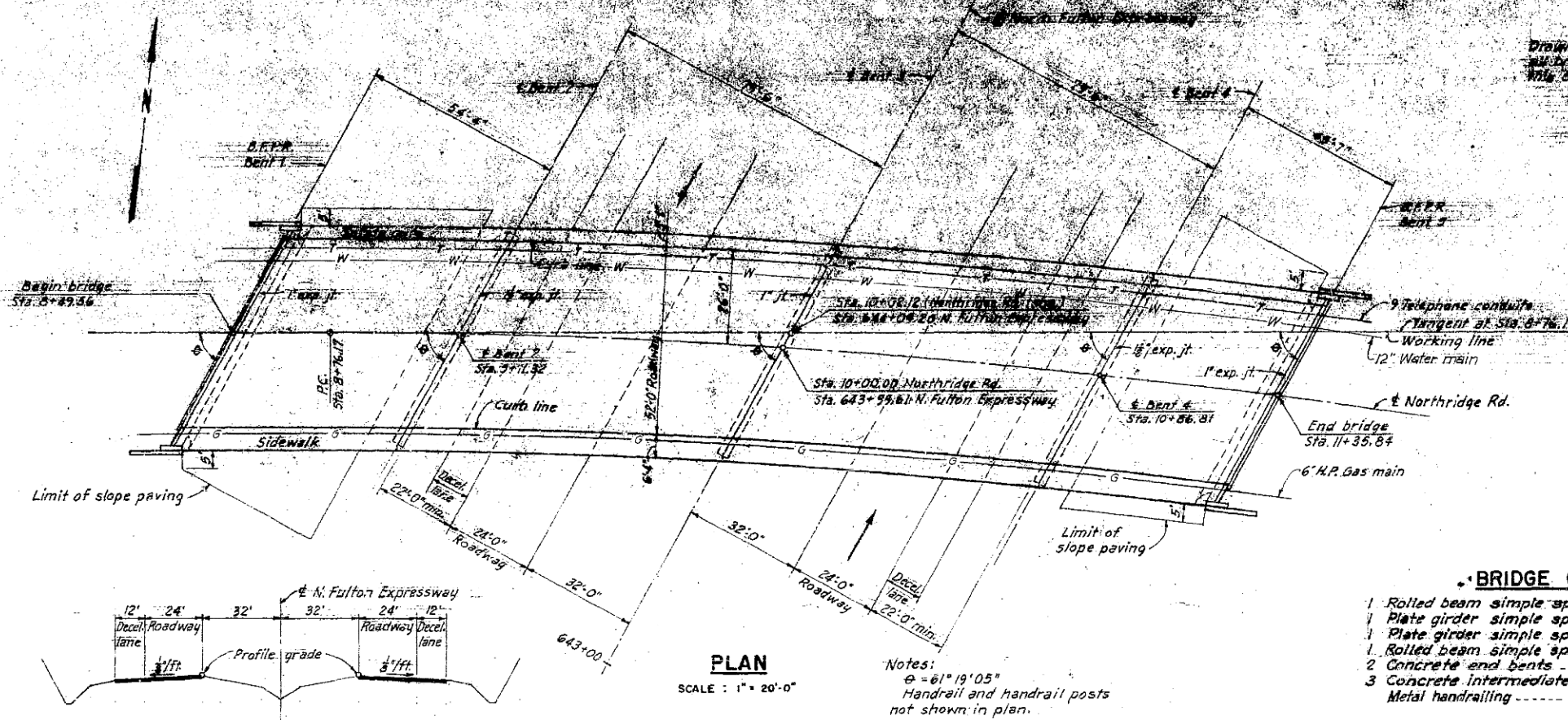
TRUE LINE AND GRADE - Roadway surface, curbs, and handrailing shall conform to true line and grade. Posts shall be vertical.

TEST PILES - All test piles shall remain in place as permanent piles.

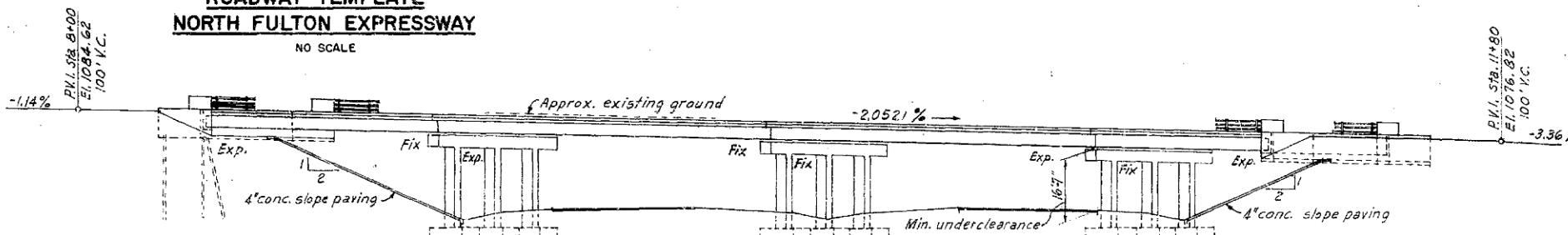
COST OF INCIDENTAL ITEMS - Cost of expansion material, elastomeric polymer sealer, weep holes, etc., and red lead and other incidental items shall be included in prices bid for contract items.

DESIGN DATA :

Specifications : AASHTO 1961, T61, T62, T63, T64.
Loading : HS 20-44 and Military Loading, Impact allowed.
Future Wearing Surface Allowance : 15 lbs. per sq. ft.



ROADWAY TEMPLATE NORTH FULTON EXPRESSWAY



HORIZONTAL CURVE DATA

NO SCALE

PROFILE NORTH FULTON EXPRESSWAY

NO SCALE

SECTION AT END BENTS

NO SCALE

ROADWAY TEMPLATE - NORTHRIDGE RD.

NO SCALE

FULTON COUNTY, GEORGIA

NORTH FULTON EXPRESSWAY

BRIDGE NO. 13

NORTHRIDGE ROAD

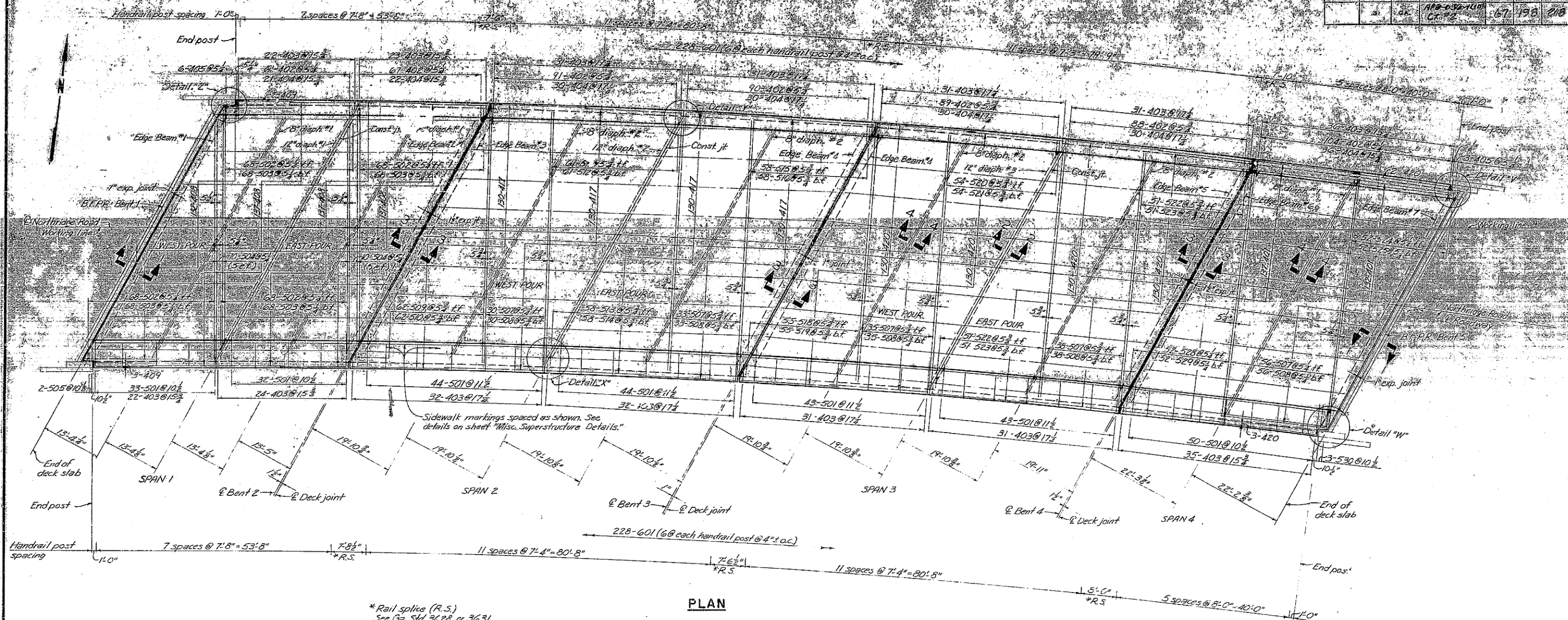
PLAN AND ELEVATION

PATCHEN, HINGLEDORFF AND ASSOCIATES
AUGUSTA, GA. ATLANTA, GA. CONSULTING ENGINEERS

DESIGNED BY H.C.D. R.A.Z. CHECKED BY H.C.D. APPROVED BY J.E. Had

SCALE: As shown DATE: Jan. 1967 DRAWING NO. 103 1 OF 13

STATE AID	REGION	STATE	FEDERAL AID	PROJECT NO.	SHEET NO.	TOTAL SHEETS
	3	GA.	100-030-100	CR-2	67	198
					218	



PLAN

* Rail splice (R.S.)
See Ga. Std. 3628 or 3631

NOTES:

- For handrail details see Ga. Std. No. 3628 or 3631.
- For section- and details not shown on deck sheets see Miscellaneous Superstructure Details.
- All 8" and 12" diaphragms shall be poured 48 hours in advance of slab.
- Slab depth between beam copings shall be uniform thickness as shown on the plans, except under sidewalk and safety curb. Necessary adjustments of deck grades to compensate for dead load and vertical curve offsets shall be made by varying slab depth over beams.
- See "Reinforcement Schedule" for steel reinforcement dimensions.
- Pours may be made in any sequence.
- Within a structural unit the 2nd Pour may not be made until the concrete in the 1st Pour has reached an age of 24 hours. This requirement may be waived if the 2nd Pour is completed except for final finishing within 4 hours of the initial placement of concrete in the 1st Pour.
- Sidewalk will be poured in the same sequence as deck pours after the last slab pour in the unit reaches a compressive strength of 1500 p.s.i. Curb pours may be made in any sequence after the last slab pour in the unit reaches a compressive strength of 1500 p.s.i.

NOTE "A":
Utility company will furnish and Contractor will set 2" CB-Universal concrete insert (Grinnell fig. 282 or equal). Space inserts a max. of 8'-0" centers in underside of slab. Utility company will furnish and install all utility hangers and will paint in accordance with Georgia Standard Specifications. Use Adjustable Wrought Clevis (Grinnell 6" fig. 260 or equal) 3/4" dia. suspension rod for 6" gas main.

FULTON COUNTY, GEORGIA			
NORTH FULTON EXPRESSWAY			
BRIDGE NO. 13			
NORTHBRIDGE ROAD			
DECK - SHEET 1 OF 3			
PATCHEN, MINGLEDORFF AND ASSOCIATES		CONSULTING ENGINEERS	
AUGUSTA, GA.		ATLANTA, GA.	
DESIGNED BY	CHECKED BY	APPROVED BY	
H.C.D. AMZ	R.A.Z.	J.E. Head	
SCALE	DATE	SHEET NO.	DRAWING NO.
3" = 32'-0"	Jan. 1967	ID3	2 OF 13

[illegible]

Technical drawing of a bridge deck cross-section. The drawing includes various structural details, dimensions, and annotations. Key features include:

- Dimensions:**
 - Overall width: 52'-0" (Radius)
 - Span 2: 28'-0"
 - Span 3: 28'-0"
 - Span 4: 6'-4"
 - Span 5: 3'-1"
 - Span 6: 6'-4"
 - Span 7: 3'-1"
 - Span 8: 6'-4"
 - Span 9: 3'-1"
 - Span 10: 6'-4"
 - Span 11: 3'-1"
 - Span 12: 6'-4"
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 - Span 255: 3'-1"
 - Span 256: 6'-4"
 - Span 257: 3'-1"
 - Span 258: 6'-4"
 - Span

[illegible]

LOOKING AHEAD .

PART ELEVATION - EDGE BEAMS NO. 1, 2, 6 & 7

SCALE : 3" = 8'-0"

The drawing is a side elevation of a structural assembly. At the top, it is labeled 'LOOKING AHEAD .'. Below this is the title 'PART ELEVATION - EDGE BEAMS NO. 1, 2, 6 & 7' in bold, underlined text. A scale of '3" = 8'-0"' is provided. The drawing shows a horizontal section with various structural elements. On the left, a vertical section is labeled 'Typ.' and '1/4"'. A horizontal section is labeled '10" 8-414 @ equal spacing Typical exterior bays'. Below this, a horizontal section is labeled '1-7" (Typ)'. A vertical section is labeled '1-803' and '8" diaphragm'. A circular detail is shown. On the right, a horizontal section is labeled '10" 8-411 @ equal spacing Typical 5' interior bays'. Below this, a horizontal section is labeled '1-803' and '1-802'. A vertical section is labeled '1-801' and '1-802'. A horizontal section is labeled '1-802'.

LOOKING AHEAD

PART ELEVATION - 8" DIAPHRAGM NO. 1

SCALE : 3" = 8'-0"

LOOKING AHEAD

PART ELEVATION - 8" DIAPHRAGM NO. 2

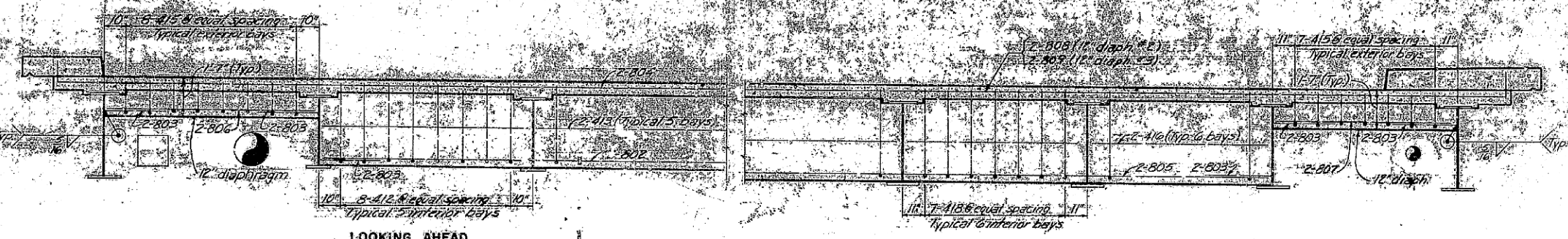
SCALE : 3" = 8'-0"

8.2 11 11 55



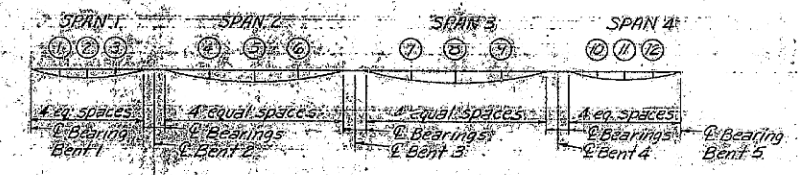
NO.	DATE	REMARKS
47	2/10	Relocate water main
55		

STATE AID	SECTION	PROJECT NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
GA	2A	100-054-100	67	200	210



LOOKING AHEAD
PART ELEVATION - 12" DIAPHRAGM NO. 1
SCALE: 3" = 8'-0"

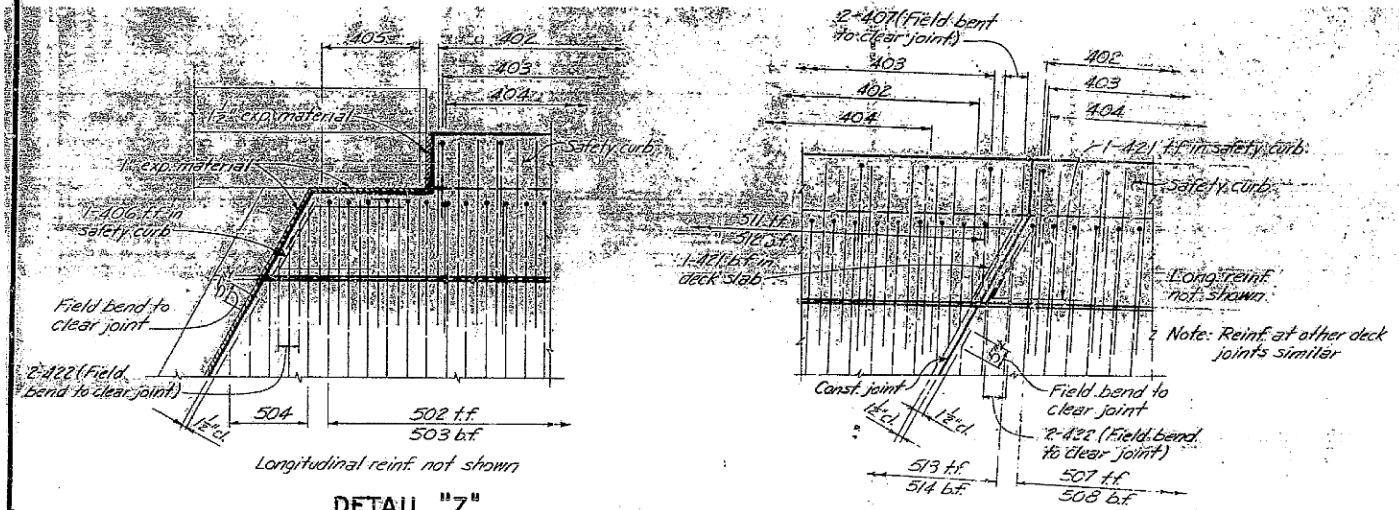
LOOKING AHEAD
PART ELEVATION - 12" DIAPHRAGMS NO. 2 & 3
SCALE: 3" = 8'-0"



DEAD LOAD DEFLECTION ORDINATES (INCHES)

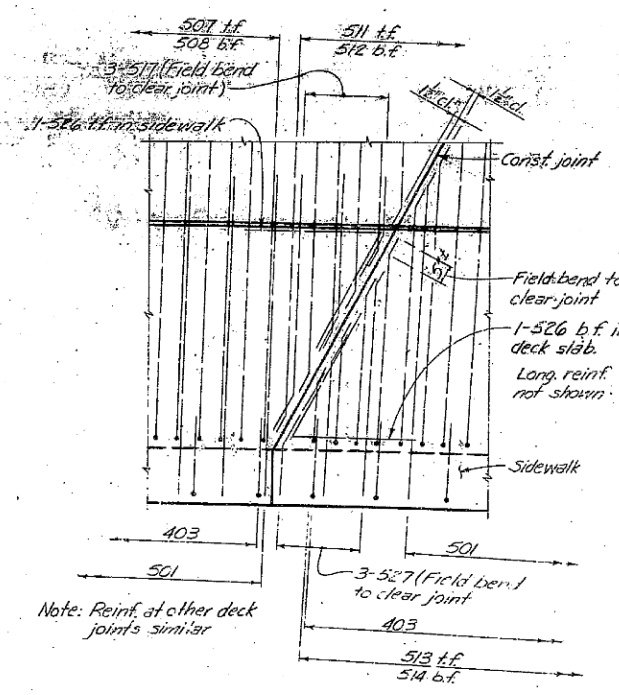
BEAM	POINT											
	1	2	3	4	5	6	7	8	9	10	11	12
A	0.49	0.64	0.49	1.31	1.84	1.31	1.19	1.67	1.19	0.25	0.36	0.25
B-H	0.67	0.94	0.67	1.27	1.77	1.27	1.25	1.75	1.25	0.30	0.42	0.30
J	0.63	0.89	0.63	1.55	2.10	1.55	1.44	2.09	1.44	0.32	0.44	0.32

Dead load deflections include dead load due to weight of slab, sidewalk, safety curb, handrail, and future paving surface.

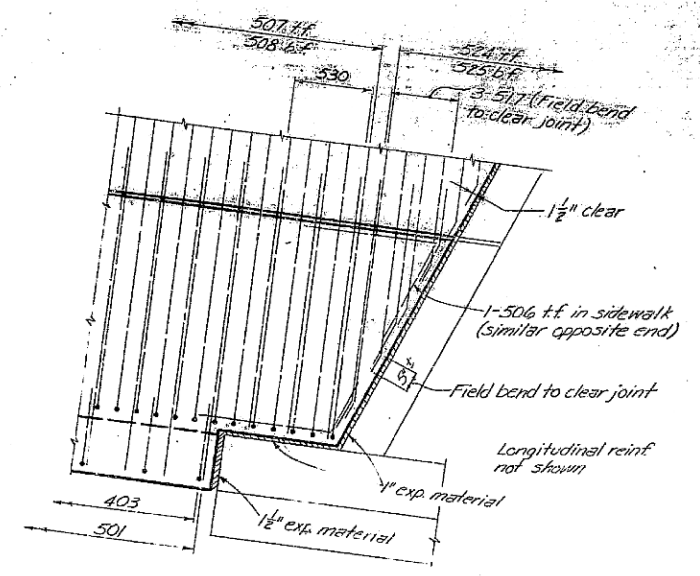


DETAIL "Z"
SCALE: 1" = 2'-0"

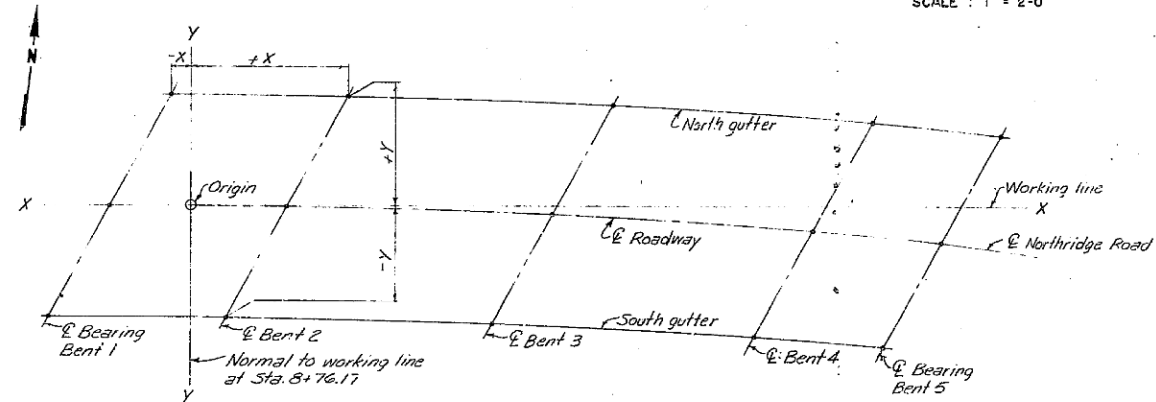
DETAIL "Y"
SCALE: 1" = 2'-0"



DETAIL "X"
SCALE: 1" = 2'-0"



DETAIL "W"
SCALE: 1" = 2'-0"



KEY PLAN
NO SCALE

DECK ELEVATIONS

POINT	NORTH GUTTER				ROADWAY				SOUTH GUTTER			
	STA.	DECK EL.	X	Y	STA.	DECK EL.	X	Y	STA.	DECK EL.	X	Y
BRG. BENT 1	8+65.78	1083.54	-10.39	26.00	8+51.55	1083.56	-24.62	0.00	8+37.33	1083.58	-38.84	-26.00
BENT 2	9+24.72	1082.33	+49.20	25.37	9+11.31	1082.34	+35.14	-0.32	8+97.49	1082.35	+21.03	-26.12
BENT 3	10+11.93	1080.54	+137.49	21.11	10+00.00	1080.52	+123.74	-4.01	9+87.71	1080.50	+109.96	-29.21
BENT 4	10+97.35	1078.79	+223.69	13.03	10+86.31	1078.73	+210.21	-11.60	10+75.95	1078.69	+196.70	-36.30
BRG. BENT 5	11+43.43	1077.83	+270.01	7.08	11+33.61	1077.77	+256.66	-17.33	11+23.51	1077.71	+243.29	-41.78

* Elevations are profile grade at Roadway
See Detail "V"

SUPERSTRUCTURE QUANTITIES

ITEM	DIAPHRAGMS	SPAN 1		SPAN 2		SPAN 3		SPAN 4	TOTAL
		WEST POUR	EAST POUR	WEST POUR	EAST POUR	WEST POUR	EAST POUR		
C-1 Vias Class A Concrete	45.7	53.9	53.9	76.8	75.9	75.1	74.4	82.7	538.4
Lbs Bar-Reinforcement Steel	6308	14,011	14,016	18,315	18,206	17,973	17,868	22,457	129,754
*Lbs Structural Steel	-	81,544	-	170,241	-	161,172	-	56,403	469,360
Lin Ft Metal Handrailing	-	-	-	-	-	-	-	-	558.17

* Includes 668 lbs. bronze (bearings) and 7476 lbs stud shear connectors.

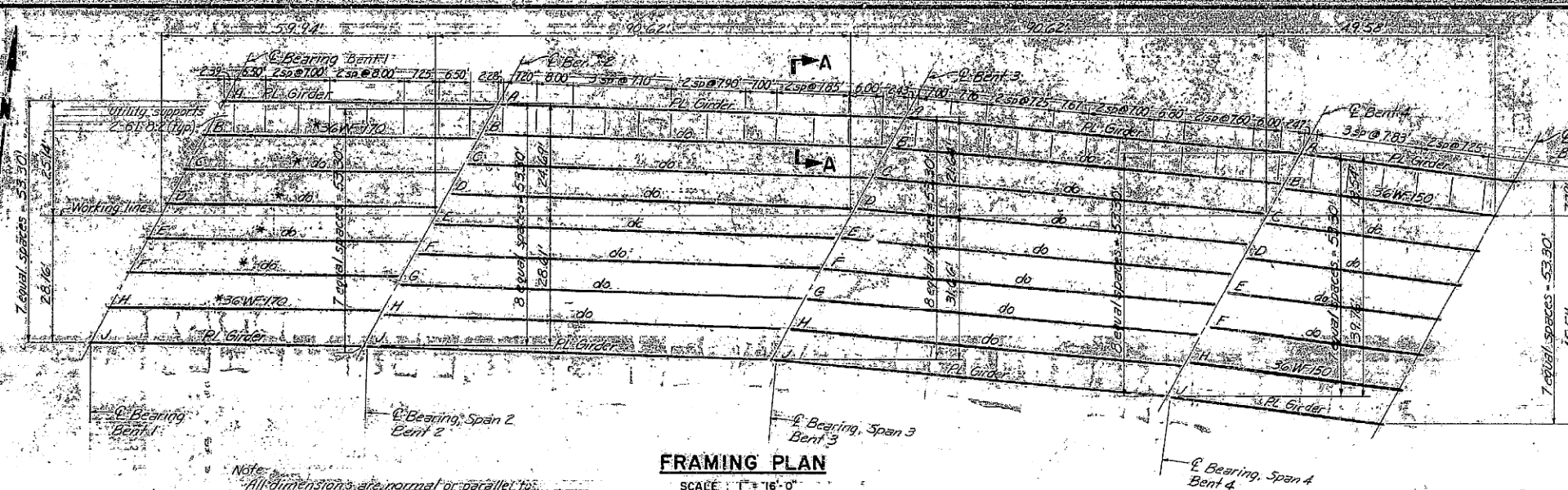
FULTON COUNTY, GEORGIA
NORTH FULTON EXPRESSWAY
BRIDGE NO. 13
NORTHBRIDGE ROAD
DECK - SHEET 3 OF 3

PATCHEN, MINGLEDORFF AND ASSOCIATES CONSULTING ENGINEERS
ATLANTA, GA.

DESIGNED BY: H.C.D. DRAWN BY: J.A.M. CHECKED BY: R.A.Z. APPROVED BY: J.E. Head

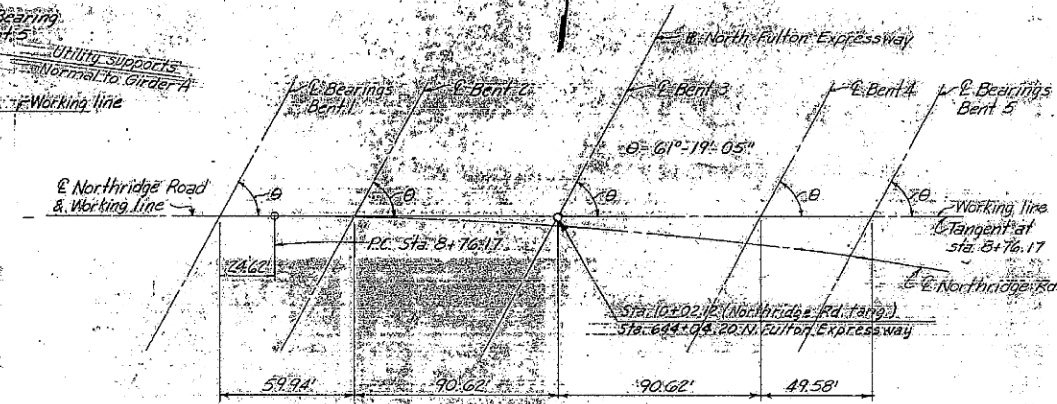
DATE: Jan. 1967 JOB NO. 103 DRAWING NO. 4 OF 13

STATE AID PROJ. No.	REGION No.	STATE	FEDERAL AID PROJECT No.	FISCAL YEAR	UNCT No.	TOTAL SHEETS
115	3	GA	770-056-100 CT 12	67	201	210



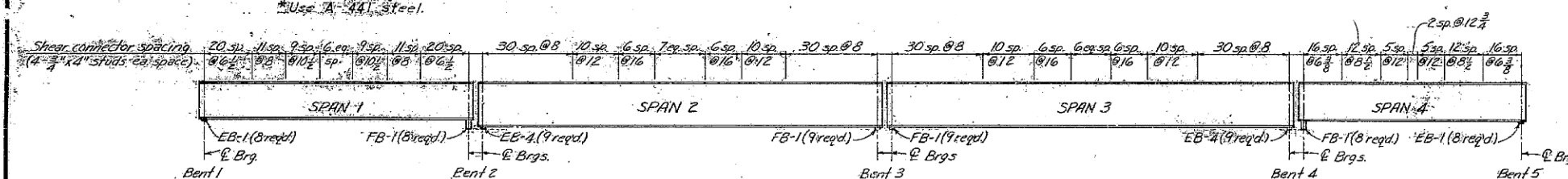
FRAMING PLAN

SCALE : 1" = 16'-0"



BRIDGE LAYOUT

NO SCALE



TYPICAL ELEVATION

NO SCALE

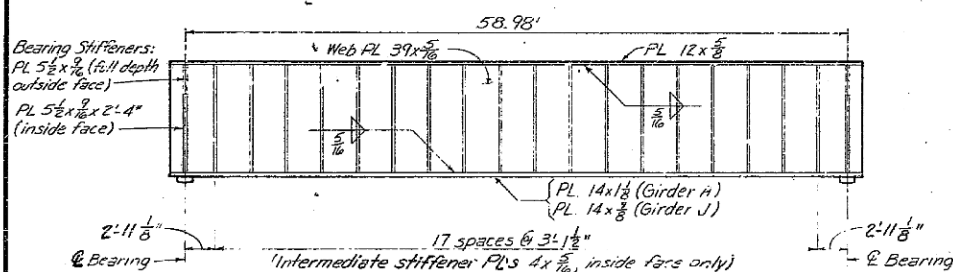


PLATE GIRDER (2 REQ'D.) - SPAN 1

NO SCALE

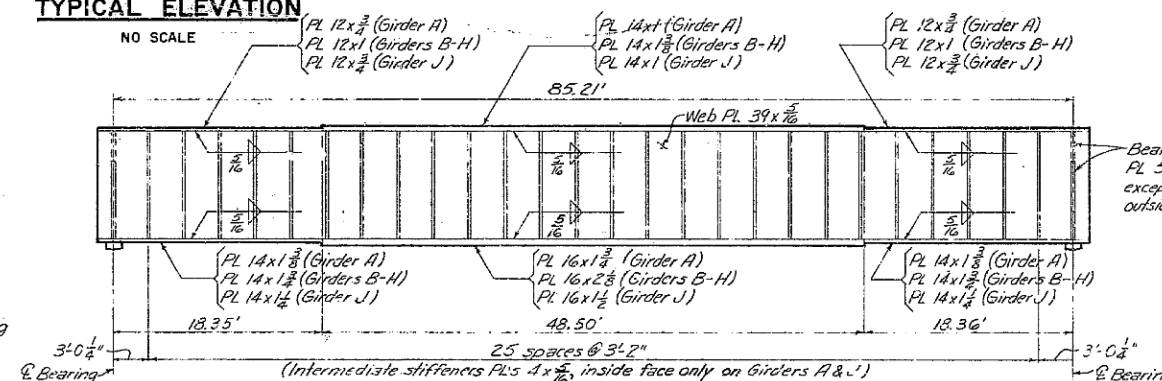


PLATE GIRDER (9 REQ'D.) - SPAN 3

NO SCALE

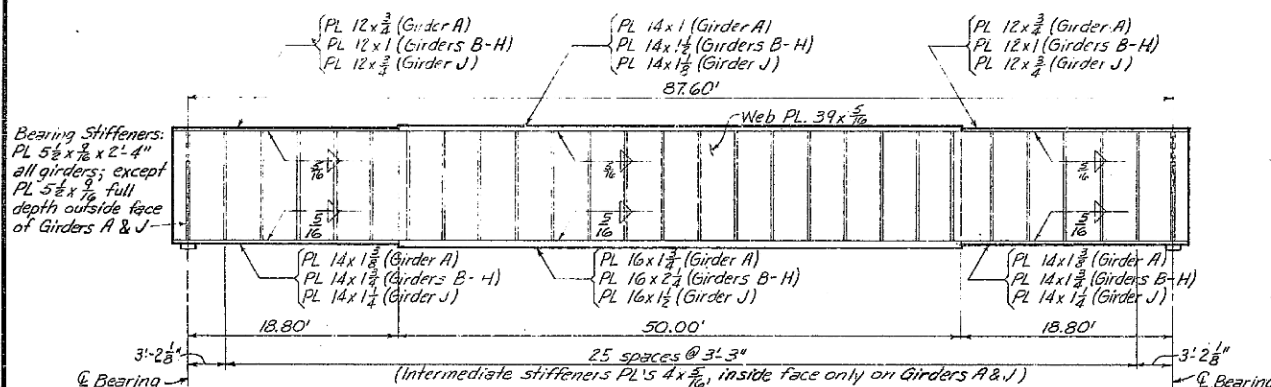


PLATE GIRDER (9 REQ'D.) - SPAN 2

NO SCALE

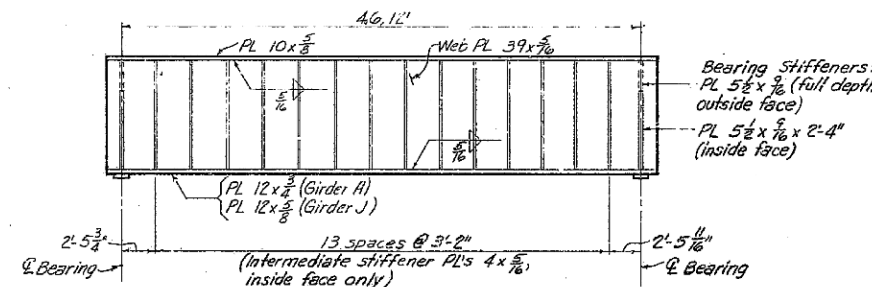
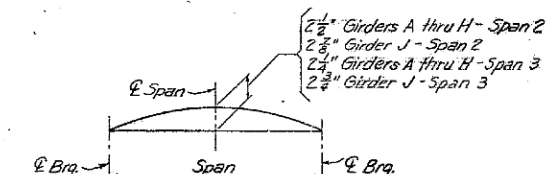


PLATE GIRDER (2 REQ'D.) - SPAN 4

NO SCALE



CAMBER DIAGRAM

Includes D.L. Deflection &
Vertical Curve Ordinates

NO SCALE

Note:
Do not camber girders
in spans 1 & 4.

Notes:
All beams and plate girders shall be A-36 steel except as noted on Framing Plan. All other structural steel shall be A-36 or A-7.
Provide holes in webs of beams and girders for reinforcement bars at edge beams and diaphragms. Cut ends of beams and plate girders vertical. Set bearing stiffeners vertical.
Turn natural camber up on rolled beams for Spans 1 & 4. For details of bearings, see sheet "Bearing Details". Camber all girders for Spans 2 & 3, as shown on Camber Diagram.
Do not camber girders or beams for Spans 1 & 4. For details of shear connectors and concealment plates see sheet "Miscellaneous Steel Details". For spacing of shear connectors, Girders A & J, Span 4, see detail on sheet "Structural Steel - Sheet E of 2."
Concrete deck must be placed before utilities are supported from utility hangers.

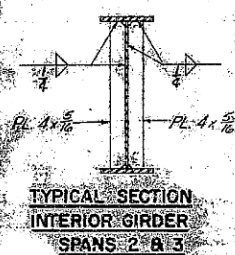
FULTON COUNTY, GEORGIA

NORTH FULTON EXPRESSWAY

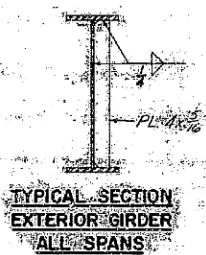
BRIDGE NO. 13

STRUCTURAL STEEL - SHEET 1 OF 2

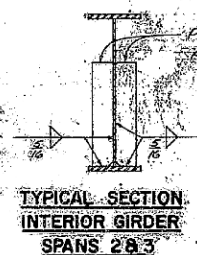
				PATCHEN, HINGLEDORFF AND ASSOCIATES				CONSULTING ENGINEERS	
				AUGUSTA, GA.				ATLANTA, GA.	
DESIGNED BY		DRAWN		CHECKED		APPROVED			
C.M.P.		A.M.Z.		H.C.D.		<i>J.E. Head</i>			
SCALE				DATE		JOB NO.		DRAWING NO.	
AS SHOWN				Jan. 1967		ID 3		5 OF 13	
1-Bldg Relocate water main utility supports									
NO.	DATE	REVISION		BY					



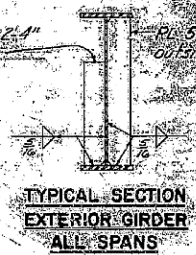
TYPICAL SECTION
INTERIOR GIRDER
SPANS 2 & 3



TYPICAL SECTION
EXTERIOR GIRDER
ALL SPANS



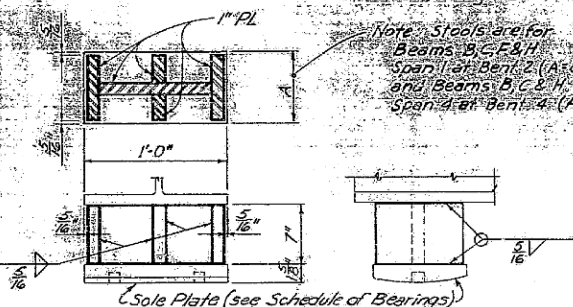
TYPICAL SECTION
INTERIOR GIRDER
SPANS 2 & 3



TYPICAL SECTION
EXTERIOR GIRDER
ALL SPANS

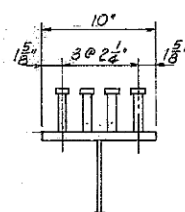
INTERMEDIATE STIFFENERS

Note:
Clip all stiffener plates 45° to
clear flange-to-web fillet welds.
The lower end of all stiffener
plates shall be ground to fit the
bottom flanges.



DETAIL OF STOOL FOR FIXED BEARINGS (FB-1)
SPAN 1 - BENT 2 (4 REQ'D); SPAN 4 - BENT 4 (3 REQ'D)

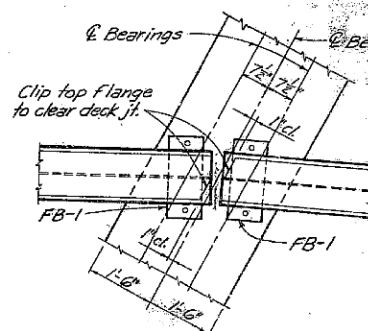
BEARING STIFFENERS



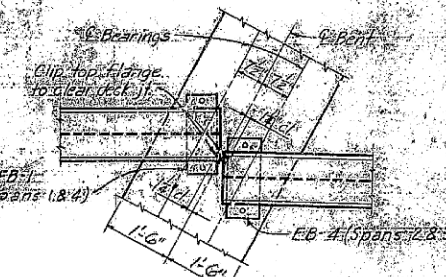
SHEAR CONNECTOR STUD SPACING
GIRDERS A & J, SPAN 4

Note: Typical spacing
for other bms. & grds.
shown on sheet
"Misc. Steel Details."

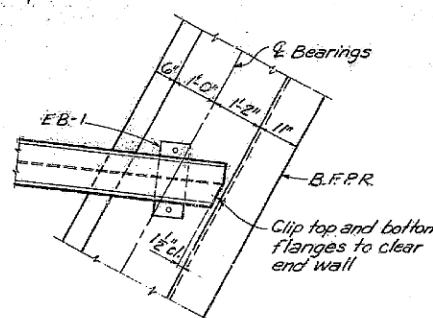
PLAN OF BEAMS AT BENT 1



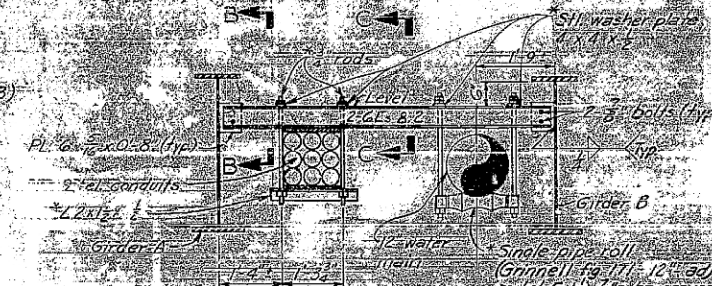
PLAN OF BEAMS AT BENT 3



PLAN OF BEAMS AT BENTS 2 & 4

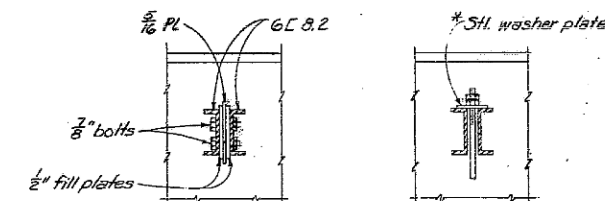


PLAN OF BEAMS AT BENT 5



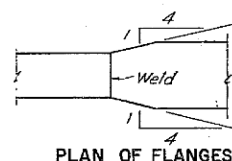
SECTION A-A TYPICAL UTILITY SUPPORT

Note:
Channels and connections to beams to be
furnished, installed and painted by Contractor.
All hangers will be furnished and set by Utility
Companies. Painting will be done by Utility
Companies in accordance with Georgia Standard
Specifications.



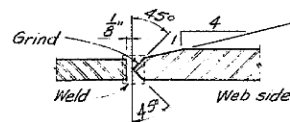
SECTION B-B

SECTION C-C TYPICAL HANGER ARRANGEMENT



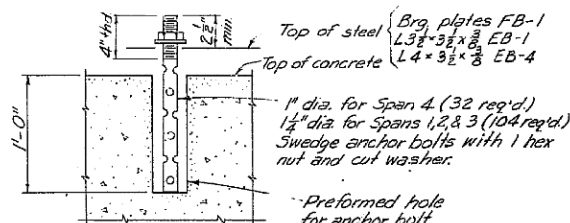
PLAN OF FLANGES

TYPICAL WEB PLATE DETAIL



TYPICAL FLANGE PLATE DETAIL

SHOP SPLICES



ANCHOR BOLT DETAIL

TOP OF STEEL ELEVATIONS

BEAM	BRG. BENT 1	BRG. BENT 2 SPAN 1	BRG. BENT 2 SPAN 2	BRG. BENT 3 SPAN 2	BRG. BENT 3 SPAN 3	BRG. BENT 4 SPAN 3	BRG. BENT 4 SPAN 4	BRG. BENT 5
A	1082.90	1081.70	1081.65	1079.89	1079.87	1078.15	1078.13	1077.19
B	1082.90	1081.70	1081.65	1079.88	1079.87	1078.13	1078.11	1077.17
C	1082.91	1081.70	1081.65	1079.88	1079.86	1078.10	1078.10	1077.15
D	1082.91	1081.70	1081.65	1079.87	1079.85	1078.10	1078.08	1077.13
E	1082.92	1081.71	1081.66	1079.86	1079.85	1078.09	1078.06	1077.11
F	1082.93	1081.71	1081.66	1079.86	1079.84	1078.08	1078.05	1077.09
G	—	—	1081.66	1079.85	1079.83	1078.06	—	—
H	1082.93	1081.71	1081.66	1079.85	1079.83	1078.05	1078.03	1077.07
J	1082.93	1081.72	1081.67	1079.84	1079.83	1078.04	1078.02	1077.06

SCHEDULE OF BEARINGS

TYPE BRG.	LOCATION	DIMENSIONS								W
		R	A	B	C	D	E	F	T	
EB-1	Bent 1 (Ords. A&J)	15"	8"	8"	9"	1 1/2"	1 1/2"	1'-10"	—	12" for 36WF170 14" for PL Girder
EB-1	Bent 1 (Ords. B-H)	15"	8"	8"	8"	1 1/2"	1 1/2"	1'-8"	—	12" for 36WF170 14" for PL Girder
FB-1	Bent 2 (Ords. A&J)	15"	8"	8"	9"	1 1/2"	1 1/2"	1'-10"	—	12" for 36WF170 14" for PL Girder
FB-1	Bent 2 (Ords. B-H)	15"	8"	8"	8"	1 1/2"	1 1/2"	1'-8"	—	12" for 36WF170 14" for PL Girder
EB-4	Bent 2 Span 2	15"	9"	9"	9"	1 1/2"	1 1/2"	1'-10"	—	12" for 36WF170 14" for PL Girder
FB-1	Bent 3 Span 2	15"	9"	9"	10"	1 1/2"	1 1/2"	2'-0"	—	12" for 36WF170 14" for PL Girder
EB-4	Bent 4 Span 3	15"	9"	9"	9"	1 1/2"	1 1/2"	1'-10"	—	12" for 36WF170 14" for PL Girder
FB-1	Bent 4 Span 4	12"	7"	7"	8"	1 1/2"	1 1/2"	1'-8"	—	12" for 36WF170 14" for PL Girder
EB-1	Bent 5	12"	7"	7"	8"	1 1/2"	1 1/2"	1'-8"	—	12" for 36WF170 14" for PL Girder

TYPE BRG.	LOCATION	THICKNESS OF BEARING PLATE "I" (INCHES)									
		A	B	C	D	E	F	G	H	J	
EB-1	Bent 1	1 1/8"	1 1/8"	1 1/8"	1 1/2"	1 1/8"	1 1/8"	—	1 1/8"	1 1/8"	
FB-1	Bent 2 Span 1	3 3/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	—	1 1/8"	3 3/8"	
EB-4	Bent 2 Span 2	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	
FB-1	Bent 3 Span 2	1 1/8"	2 1/8"	2 1/8"	2 1/8"	2 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	
EB-4	Bent 3 Span 3	1 1/8"	2 1/8"	2 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	
EB-4	Bent 4 Span 3	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	
FB-1	Bent 4 Span 4	3 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	—	1 1/8"	3 1/8"	
EB-1	Bent 5	1 1/8"	1 1/8"	1 1/8"	2 1/8"	1 1/8"	1 1/8"	—	1 1/8"	1 1/8"	

FULTON COUNTY, GEORGIA
NORTH FULTON EXPRESSWAY
BRIDGE NO. 13
NORTHRIDGE ROAD
STRUCTURAL STEEL - SHEET 2 OF 2

PATCHEN, MINGENDORFF AND ASSOCIATES CONSULTING ENGINEERS
AUGUSTA, GA. ATLANTA, GA.

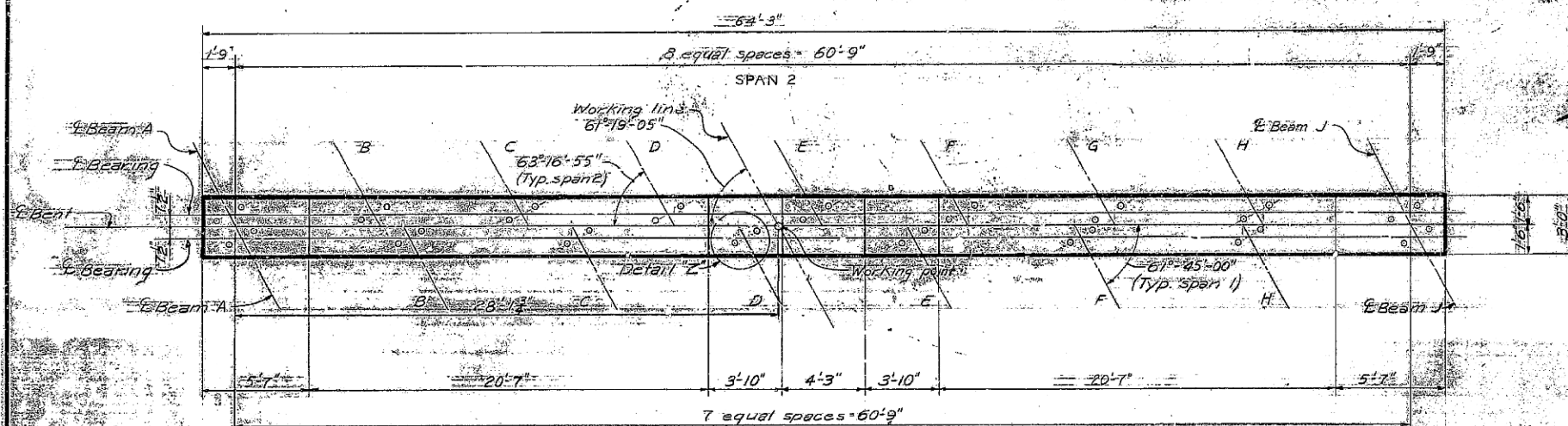
DESIGNED BY G.M.P. DRAWN BY J.M.Z. CHECKED BY H.C.D. APPROVED BY J.E. Head

SCALE: No scale DATE: Jan. 1967 SUB NO: 103 DRAWING NO: 6 OF 13

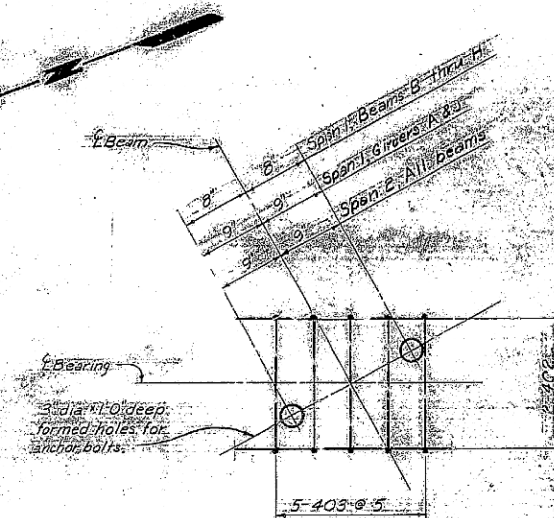
STATE AID PROJECT NO.	REGION NO.	STATE	FEDERAL AID PROJECT NO.	FISCAL YEAR	PERCENTAGE	TOTAL SHEETS
	3	GA.	APD-446-109	67	20%	218

QUANTITIES	
Concrete	55.8
Reinforcement Steel	74.425
Bridge Excavation	26

Quantities are for estimating purposes only.

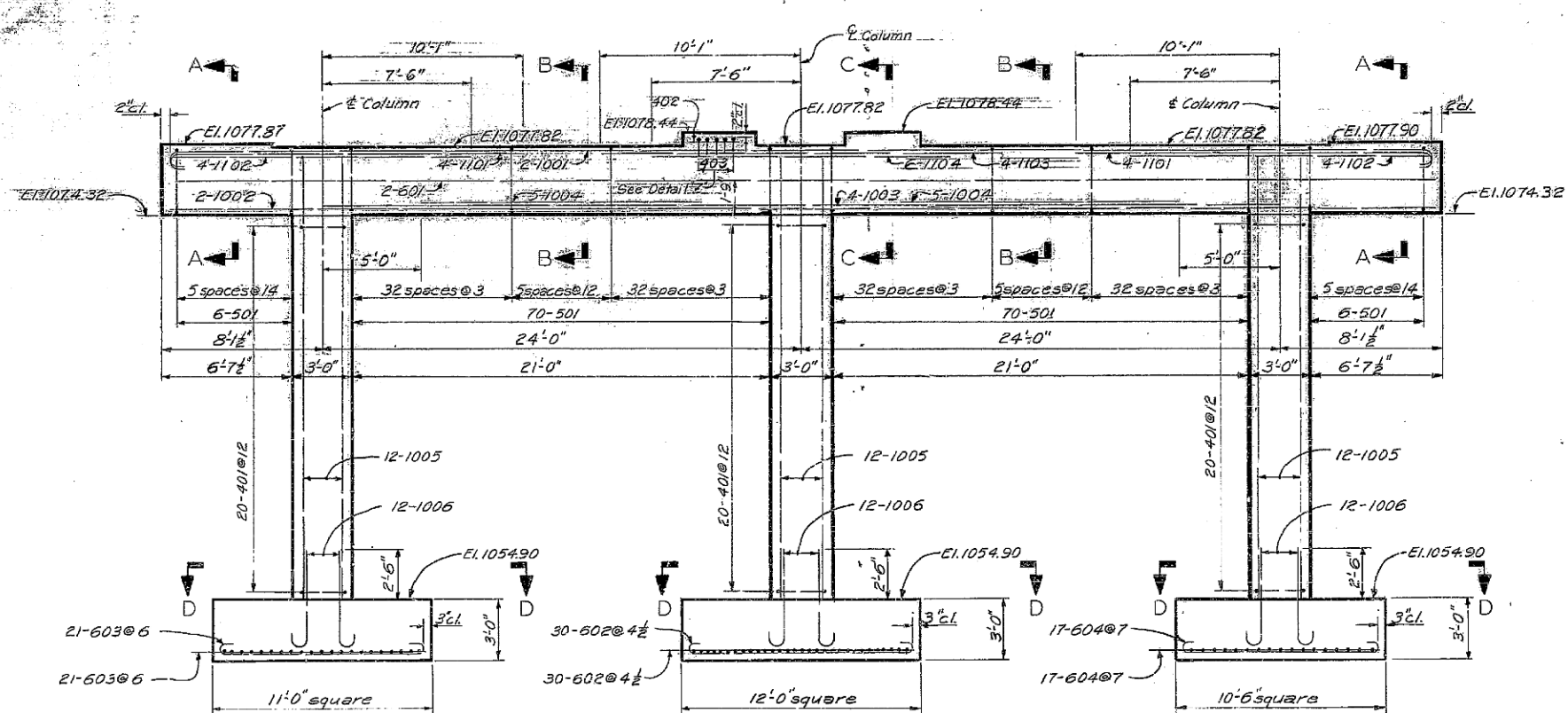


SPAN 1
PLAN OF CAP
SCALE: 1" = 4'-0"

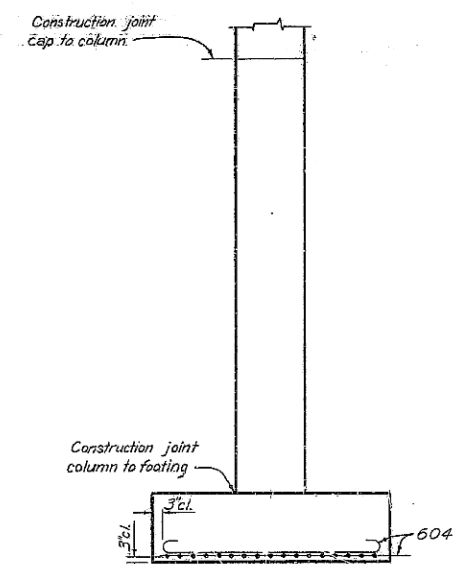


DETAIL 2

ANCHOR BOLT HOLES TYPICAL FOR ALL BEAMS AND GIRDERS AS NOTED.
REINFORCEMENT ONLY FOR BEAMS D AND E - SPAN 1.
NO SCALE

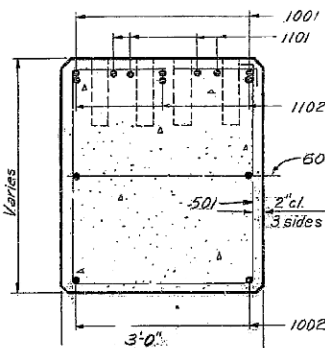


LOOKING AHEAD
FRONT ELEVATION
SCALE: 1" = 4'-0"

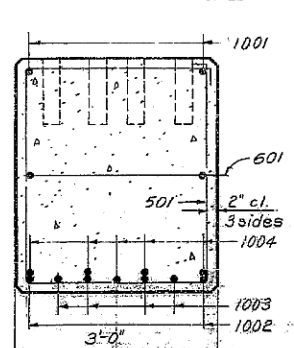


SIDE ELEVATION
SCALE: 1" = 4'-0"

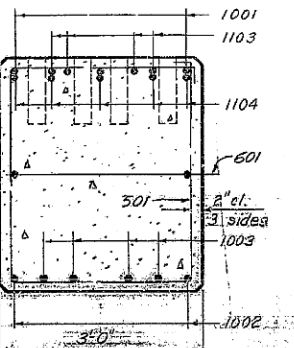
Notes:
For location of Bent No. 2 see Bridge Layout on sheet "Bent No. 1."
For Bundling Detail see sheet "Misc. General Details."
See "Reinforcement Schedule" for steel reinforcement dimensions.
Maximum soil pressure - 3 tons per sq. ft.



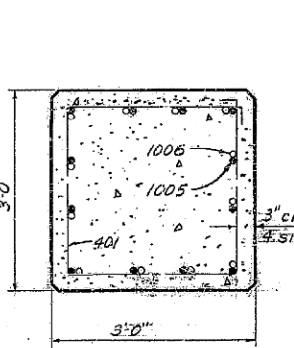
SECTION A-A
SCALE: 3" = 4'-0"



SECTION B-B
SCALE: 3" = 4'-0"



SECTION C-C
SCALE: 3" = 4'-0"



SECTION D-D
SCALE: 3" = 4'-0"

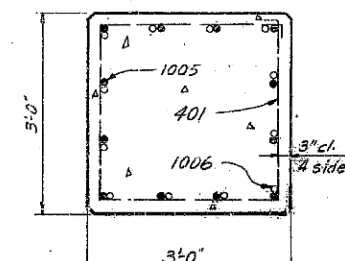
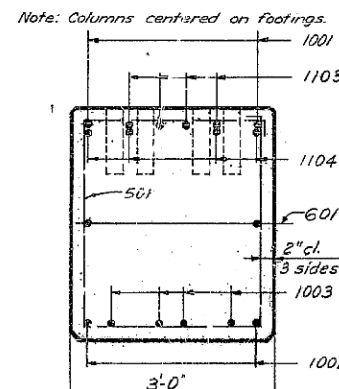
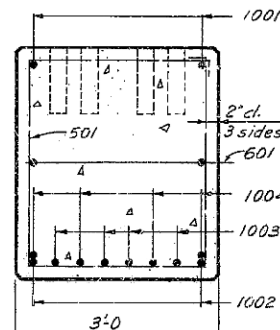
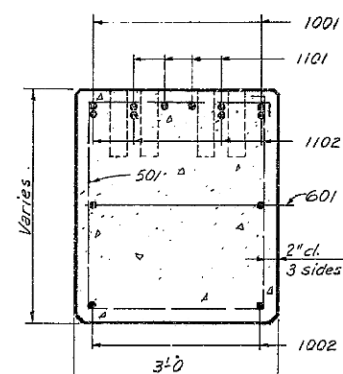
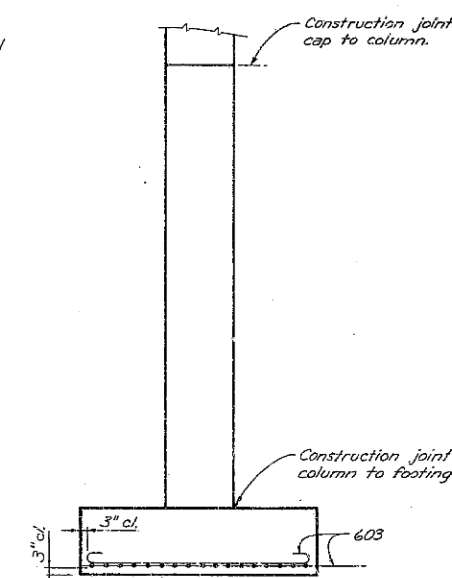
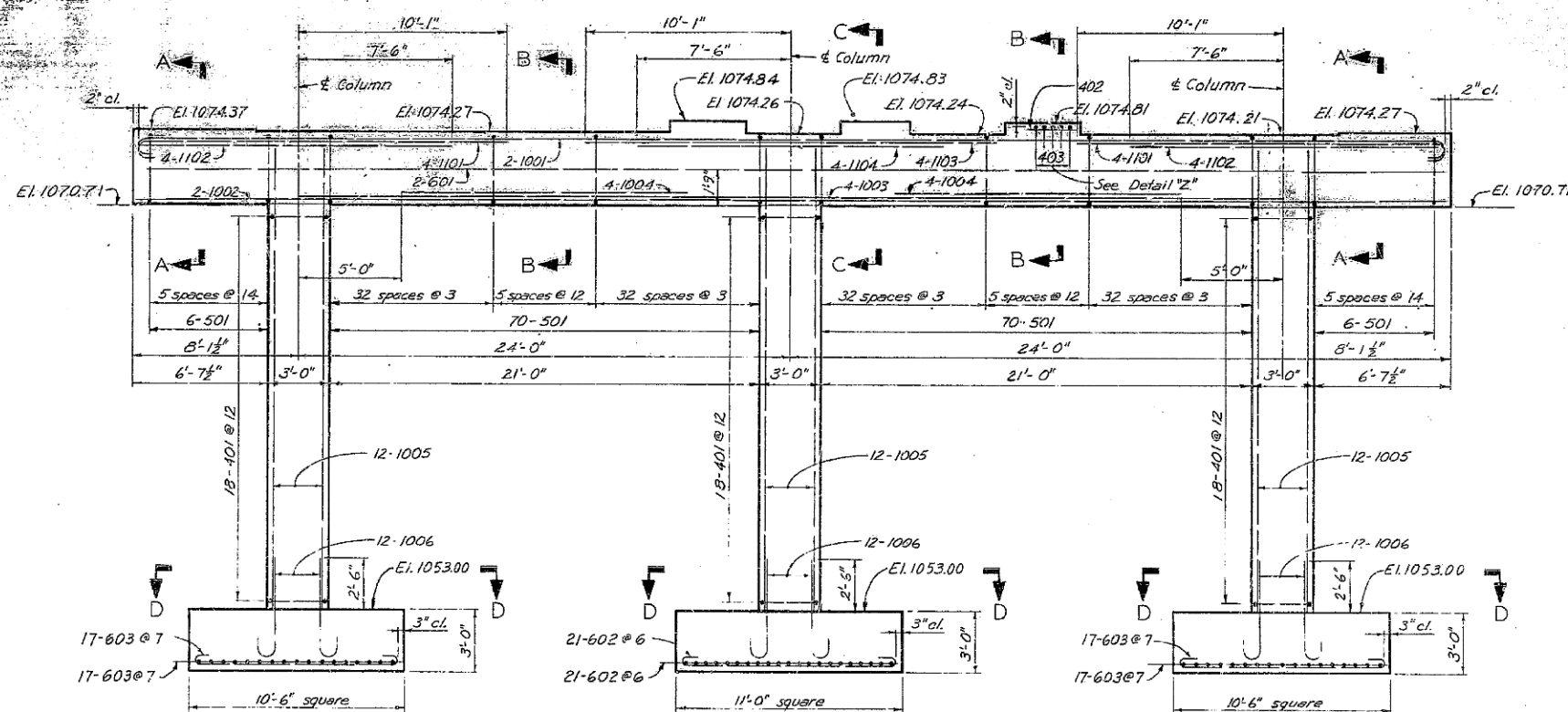
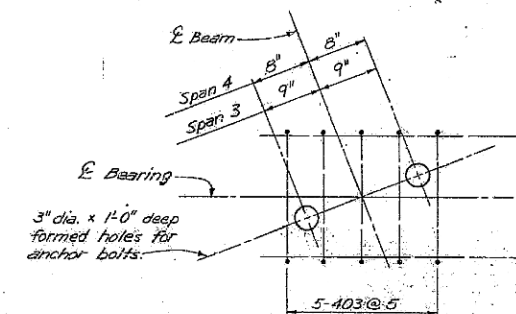
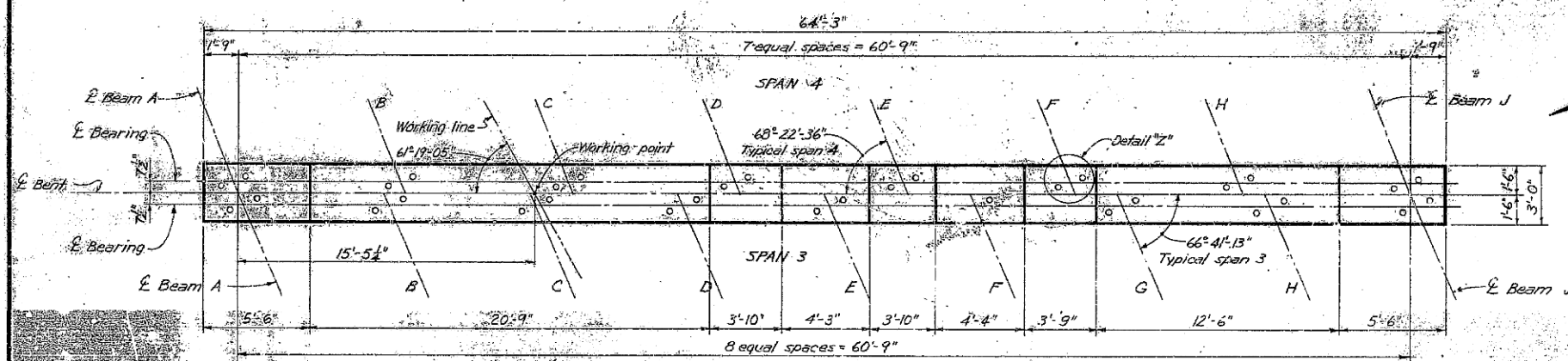
FULTON COUNTY, GEORGIA
NORTH FULTON EXPRESSWAY
BRIDGE NO. 13
NORTH RIDGE ROAD
BENT NO. 2

PATCHEN, NINGLEDORFF AND ASSOCIATES		CONSULTING ENGINEERS	
DESIGNED: HOU	CHAWN: CDS	CHECKED: RAZ	APPROVED: J.E. Hoad
SCALE: As Shown	DATE: Jan. 1967	JOB NO: 103	DRAWING NO: 8 OF 13

STATE AID	FED. AID	STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
		GA.	APD-056-160	67	206	216

QUANTITIES		
Class A Concrete	Cu. Yds.	81.7
Bar Reinforcement Steel	Lbs.	13,285
Bridge Excavation	Cu. Yds.	151

*Quantities are for estimating purposes only.



Notes:
For location of bent No. 4 see Bridge Layout on sheet "Bent No. 1"
For Bundling Detail see sheet "Misc. General Details"
See "Reinforcement Schedule" for steel reinforcement dimensions.
Maximum soil pressure = 3 tons per sq. ft.

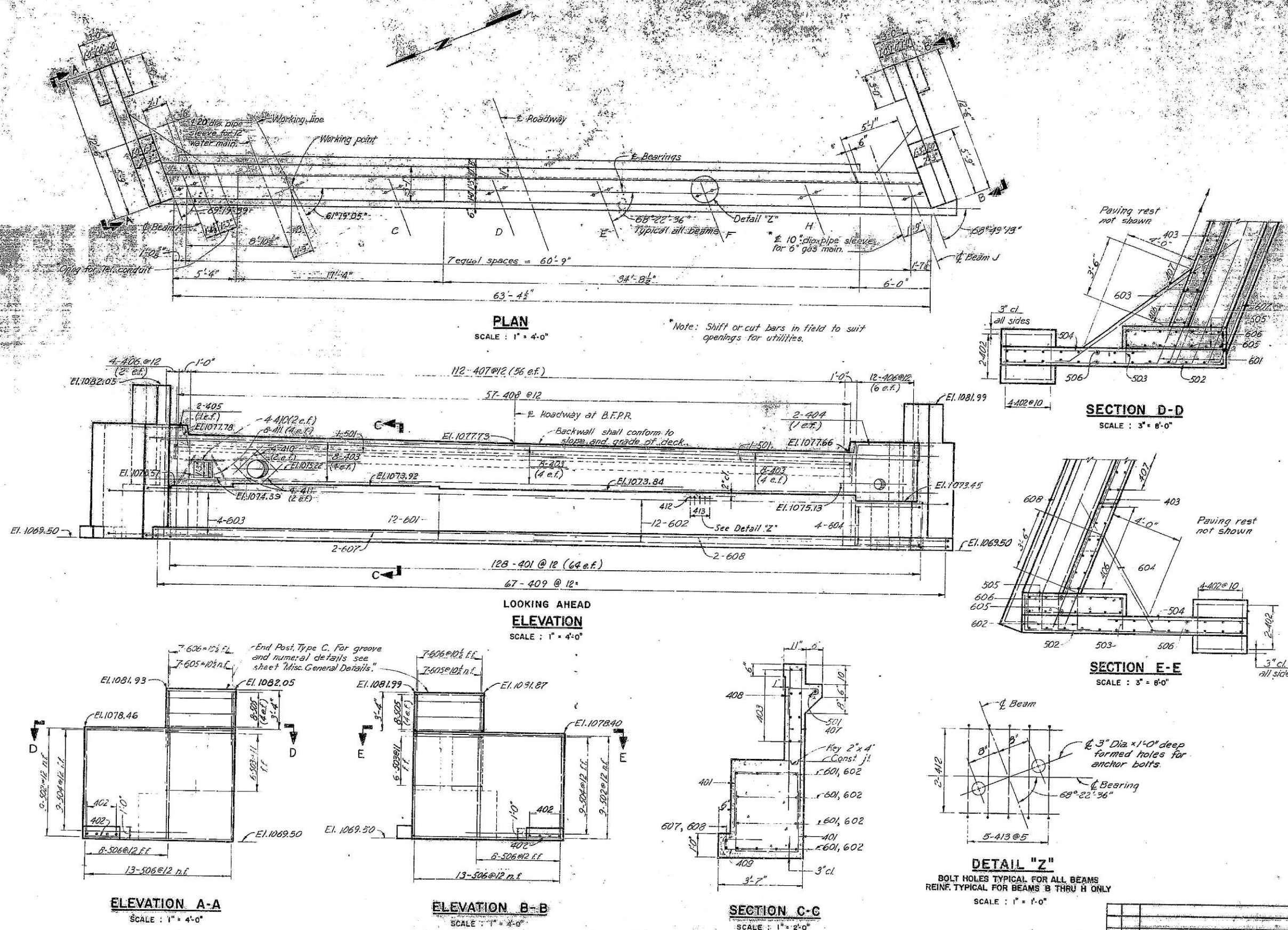
FULTON COUNTY, GEORGIA			
NORTH FULTON EXPRESSWAY			
BRIDGE NO. 13			
NORTHDRIDGE ROAD			
BENT NO. 4			
PATCHEN, WINGLEDORFF AND ASSOCIATES, CONSULTING ENGINEERS			
DESIGNED: HDU	DRAWN: JKB	CHECKED: R.A.Z.	APPROVED: J.E. Had
SCALE: As shown	DATE: Jan. 1967	SHEET NO. 103	DRAWING NO. 10 OF 13

STATE AID PROJECT NO.	REGION NO.	STATE NO.	FEDERAL AID PROJECT NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
	3	CT	RD-684-100 C-2	67	207	210

QUANTITIES

Class A Concrete	Cu Yds	59.0
Bar Reinforcement Steel	Lbs	5,134
Bridge Excavation	Cu Yds	140

*Quantities are for estimating purposes only



FULTON COUNTY, GEORGIA			
NORTH FULTON EXPRESSWAY			
BRIDGE NO. 13			
NORTHBRIDGE ROAD			
BENT NO. 5			
PATCHEN, MINGLEDORFF AND ASSOCIATES, INC.		CONSULTING ENGINEERS	
DESIGNED BY	DRAWN BY	CHECKED BY	APPROVED BY
G.M.P.	W.D.Z.	R.A.Z.	J.E.H.
DATE	DATE	DATE	DATE
Jan 1967	Jan 1967	Jan 1967	Jan 1967
SHEET NO.		DRAWING NO.	
11 OF 13		11 OF 13	

[illegible]

LOCATION	MARK	LENGTH FT. IN.	BARS REQ'D.	TYPE	A FT. IN.	B FT. IN.	C FT. IN.	D FT. IN.	E FT. IN.	F FT. IN.	G FT. IN.	H FT. IN.	N NO. OR DEGREES	O DEGREES
Bent 2	401	10 4	60	25	2 6	2 6	4							
	402	2 3	4	1										
	403	3 4	10	2	1 6	1 0	1 0							
	501	11 11	152	25	2 8	3 2	4							
	601	63 9	2	1										
	602	12 10	60	16	11 6									
	603	11 10	42	16	10 6									
	604	11 4	34	16	10 0									
	1001	66 7	2	16	63 9									
	1002	63 9	2	1										
	1003	48 0	4	1										
	1004	14 0	10	1										
	1005	22 3	36	1										
	1006	6 7	36	15	5 2									
	1101	19 8	8	15	18 1									
	1102	15 6	8	1										
	1103	20 2	4	1										
Bent 2	1104	15 0	6	1										

Note :
Work with Georgia Standard No. 3900
(Standard Bar Bending Details)
Bars marked "Ties" or "Stirrups" to be bent
with pin diameter = 2 x bar diameter.

FULTON COUNTY, GEORGIA

NORTH FULTON EXPRESSWAY

BRIDGE NO. 13

NORTHBRIDGE ROAD

REINFORCEMENT SCHEDULE-SH. 1 OF 2

PATCHEN, MINGLEDORFF AND ASSOCIATES

AUGUSTA, GA. ATLANTA, GA. CONSULTING ENGINEERS

DESIGNED: AUGUSTA, GA.

DRAWN: M.S.H. R.A.Z.

CHECKED: J.E. NASH

APPROVED: J.E. NASH

SCALE: _____

DATE: _____

JOB NO. _____

DRAWING NO. _____

Note :
 Work with Georgia Standard No. 3900
 (Standard Bar Bending Details)
 Bars marked "Ties" or "Stirrups" to be bent
 with pin diameter = $2 \times$ bar diameter.

FULTON COUNTY, GEORGIA			
NORTH FULTON EXPRESSWAY			
BRIDGE NO. 13			
NORTHDRIDGE ROAD			
REINFORCEMENT SCHEDULE-SH. 1 OF 2			
PATCHEN, MINGLEDORFF AND ASSOCIATES			CONSULTING
AUGUSTA, GA.			ENGINEERS
DESIGNED	DRAWN	CHECKED	APPROVED
	M.S.H. R.A.Z.		<i>J.E. Hard</i>
SCALE	DATE	JOB NO.	DRAWING NO.
	Jan. 1967	1 D 3	12 OF 13

ATTACHMENT 6

EXISTING BRIDGE FOUNDATION INVESTIGATION



BRIDGE FOUNDATION INVESTIGATION

PR-5610 D Fulton

Northridge Road Over North Fulton Expressway

121-0270-0

121-09260M - 005.23M

1. Location -

12

This site is located northeast of Roswell at the intersection of the North Fulton Expressway and Northridge Road. It is geologically sited in the Carolina formation of the Georgia Piedmont.

2. Subsurface Conditions -

Reference should be made to the attached boring logs. Some pertinent details are as follows:

- a. All borings reached refusal of a power auger on rock.
- b. The underlying rock at the site is quite irregular in depth and density.
- c. Hard rock is overlain by layers of weathered rock and dense soil.
- d. The ground water was not encountered at this site.

3. Foundation Recommendations -

Bent 1 - A pile bent using steel "H" piles is recommended for this bent. Maximum recommended bearing for piles is as follows:

10 BP 42	55 Tons
12 BP 53	70 Tons

Piles may be driven to bearing as determined by the dynamic driving formula, but reference should be made to the boring logs during driving to insure that pile points are seated in weathered rock.

Bent 2 - Spread footings are recommended for this bent. These footings should be placed at or below elevation 1052.5 with a design bearing not to exceed 6 kips per square foot.

Bent 3 - Spread footings are recommended at this bent. Footings should be placed at or below elevation 1054.0 with a design bearing not to exceed 6 kips per square foot.

Bent 4 - Spread footings are recommended at this bent. Footings should be placed at or below elevation 1052.0 with a design bearing not to exceed 6 kips per square foot.

BRIDGE FOUNDATION INVESTIGATION

PR-5610 D Fulton
Northridge Road Over North Fulton Expressway
Page 2

Bent 5-- Spread footings are recommended for this bent. Footings should be placed at or below elevation 1072.0 with a design bearing not to exceed 4 kips per square foot.

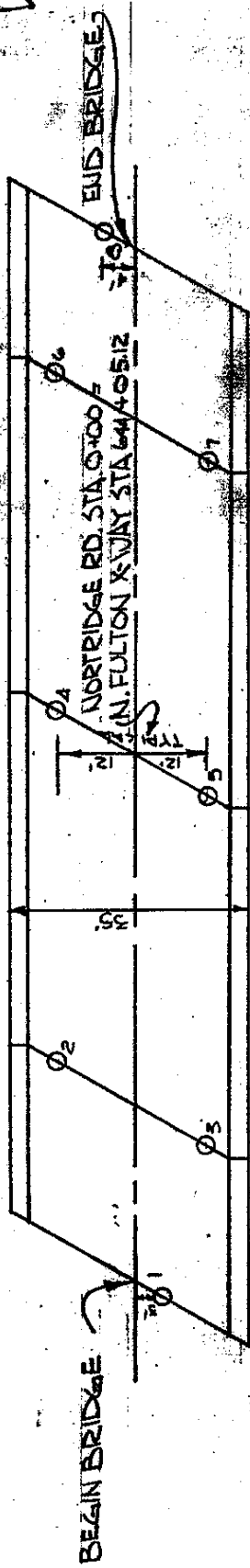
4. Danger From Fill Settlement -

No danger.

Thomas D. Moreland
Highway Materials Engineer

TDM:sev

10-20-65



39' 80' 75' 40' 234' 49.03

61.76

86.81

88.68

N.S.
1076.10
S.S.
1076.80

STATE HIGHWAY DEPARTMENT OF GEORGIA	
Bridge Foundation Investigation	
DRW. PR-5610-B	FULTON CO.
P	NORTRIDGE ROAD OVER NORTH FULTON EXPRESSWAY
CHKD. LM	
APP. TDM	
NO SCALE	10-11-65

BRIDGE SUBSURFACE INVESTIGATION

PROPOSED FOOTING ELEV. 1080.0 ± PARTY CHIEF PORTER

3' OFFSET - RIGHT

[illegible]

BRIDGE SUBSURFACE INVESTIGATION

PROPOSED FOOTING ELEV. 1052.00 ± PARTY CHIEF PORTER

NO SAMPLES - POWER LINES

[illegible]

BRIDGE SUBSURFACE INVESTIGATION

PROPOSED FOOTING ELEV. 1052.00 PARTY CHIEF PORTER

NO SAMPLES

[illegible]

BRIDGE SUBSURFACE INVESTIGATION

PROPOSED FOOTING ELEV. 1052.00 ± PARTY CHIEF PORTER

NO SAMPLES

[illegible]

STATE HIGHWAY DEPARTMENT OF GEORGIA

DIVISION OF MATERIALS AND TEST, ATLANTA, GEORGIA

BRIDGE SUBSURFACE INVESTIGATION

PROJECT PR-5610 D COUNTY FULTON DATE 9-23-65

LOCATION NORTHRIDGE RD. OVER N. FULTON EXBORING NO. 5

BENT NO. 3 FOOTING RIGHT GROUND ELEV. 1081.20

PROPOSED FOOTING ELEV. 1052.00 PARTY CHIEF PORTER

NO SAMPLES

[illegible]

BRIDGE SUBSURFACE INVESTIGATION

PROPOSED FOOTING ELEV. 1052.00 ± PARTY CHIEF PORTER

NO SAMPLES

[illegible]

STATE HIGHWAY DEPARTMENT OF GEORGIA

DIVISION OF MATERIALS AND TEST, ATLANTA, GEORGIA

BRIDGE SUBSURFACE INVESTIGATION

PROJECT PR-5610 D COUNTY FULTON DATE 9-24-65

LOCATION NORTHRIDGE ROAD OVER N. FULTON EXP. BORING NO. 7

BENT NO. 4 FOOTING RIGHT GROUND ELEV. 118.43

PROPOSED FOOTING ELEV. 1052.00 ± PARTY CHIEF PORTER

NO SAMPLES - REFUSAL ABOVE FTG.

[illegible]

BRIDGE SUBSURFACE INVESTIGATION

PROPOSED FOOTING ELEV. 1073 ± PARTY CHIEF PORTER

4' OFFSET - LEFT

[illegible]

"AS-BUILT" BRIDGE FOUNDATION REPORT

DATA FOR BRIDGE ENGINEER, ATLANTA

PROJECT NO. APP 056-1 (10) CT. 2 COUNTY FULTONBRIDGE NO. 13 (Lt.) (Rt.) OVER/AT NORTH BRIDGE AT STA. 6 + 4.9DATE OF THIS REPORT JANUARY 3 1969 BY P. E. BAINES JR. ENGINEER

ELEVATIONS OF BOTTOMS OF FOOTINGS (= BF) & LOWEST AND HIGHEST ELEVATIONS OF PILE TIPS (= LOW, HIGH)

(Express this data in whole numbers only - NO DECIMALS)

BENT NO.	FOR FOUNDATION BENTS ONLY					FOR PILE BENTS ONLY		REMARKS, INCLUDING SURVEY ORIENTATION
	ELEVATION OF	AT			LOW	HIGH		
		LEFT FOOTING	MIDDLE FOOTING	RIGHT FOOTING				
1	BF		7076				END BENT	
	LOW HIGH		1042 1069					
2	BF	1051	1051	1051			SPREAD PILE	
	LOW HIGH							
3	BF	1050	1050	1050			" "	
	LOW HIGH							
4	BF	1050	1050	1050			" "	
	LOW HIGH							
5	BF		1069				END BENT	
	LOW HIGH							
6	BF							
	LOW HIGH							
7	BF							
	LOW HIGH							
8	BF							
	LOW HIGH							
9	BF							
	LOW HIGH							
10	BF							
	LOW HIGH							
11	BF							
	LOW HIGH							
12	BF							
	LOW HIGH							
13	BF							
	LOW HIGH							
14	BF							
	LOW HIGH							
15	BF							
	LOW HIGH							
16	BF							
	LOW HIGH							
17	BF							
	LOW HIGH							
18	BF							
	LOW HIGH							
19	BF							
	LOW HIGH							
20	BF							
	LOW HIGH							
21	BF							
	LOW HIGH							
22	BF							
	LOW HIGH							

ORIGINAL TO: BRIDGE ENGINEER
COPY TO : DIVISION OFFICE

NOTE: COMPLETE THIS FORM FOR EVERY BRIDGE AS SOON AS THE INFORMATION IS KNOWN AND MAIL PROMPTLY.

ATTACHMENT 7

BRIDGE INVENTORY DATA LISTING



Bridge Inventory Data Listing



Parameters: Bridge Serial Num

Structure ID:121-0270-0

Fulton

SUFF. RATING: 54.45

Location & Geography

Structure ID: 121-0270-0
 200 Bridge Information: 06
 *6A Feature Int: SR 400 (US 19)
 *6B Critical Bridge: 0
 *7A Route No Carried: CR00145
 *7B Facility Carried: NORTHRIDGE ROAD
 9 Location: 5.2 MI N OF I-285
 2 Dot District: 7

 207 Year Photo: 2009
 *91 Inspection Frequency: 24 Date: 01/29/2009
 92A Fract Crit Insp Freq: 0 Date: 02/01/1901
 92B Underwater Insp Freq: 0 Date: 02/01/1901
 92C Other Spc. Insp Freq: 0 Date: 02/01/1901
 * 4 Place Code: 68516
 *5 Inventory Route(O/U): 1
 Type: 5
 Designation: 1
 Number: 09260
 Direction: 0
 *16 Latitude: 33 58.9320 HMMS Prefix:00
 *17 Longitude: 84 -20.4620 HMMS Suffix:000 MP:0.00
 98 Border Bridge: 000%Shared:00
 99 ID Number: 0000000000000000
 *100 STRAHNET: 0
 12 Base Highway Network: 1
 13A LRS Inventory Route: 1212014500
 13B Sub Inventory Route: 0
 101 parallel Structure: N
 *102 Direction of Traffic: 2
 *264 Road Inventory Mile Post: 000.80
 *208 Inspection Area: 7 Initials: EFP
 Engineer's Initials: sgm
 * Location ID No: 121-09260M-005.23N

*104 Highway System: 0
 *26 Functional Classification: 16
 *204 Federal Route Type: M No: 09260
 105 Federal Lands Highway: 0
 *110 Truck Route: 0
 2006 School Bus Route: 1
 217 Benchmark Elevation: 0000.00
 218 Datum: 0
 *19 Bypass Length: 04
 *20 Toll: 3
 *21 Maintanance: 01
 *22 Owner: 01
 *31 Design Load: 6
 37 Historical Significance: 5
 205 Congressional District: 06
 27 Year Constructed: 1968
 106 Year Reconstructed: 0000
 33 Bridge Medium: 0
 34 Skew: 30
 35 Structure Flared: 0
 38 Navigation Control: N
 213 Special Steel Design: 0
 267 Type of Paint: 2
 *42 Type of Service On: 5
 Type of Service Under: 1
 214 Movable Bridge: 0
 203 Type Bridge: Z
 259 Pile Encasement: 3
 *43 Structure Type Main: 3 02
 45 No.Spans Main: 004
 44 Structure Type Appr: 0 00
 46 No Spans Appr: 0000
 226 Bridge Curve Horz: 1 Vert: 0
 111 pier Protection: 0
 107 Deck Structure Type: 1
 108 Wearing Structure Type: 1
 Membrane Type: 0
 Deck Protection: 8

Signs & Attachments

225 Expansion Joint Type: 15
 242 Deck Drains: 0
 243 Parapet Location: 0
 Height: 0
 Width: 0
 238 Curb Height: 1
 Curb Material: 1
 239 Handrail: 7 7
 *240 Medium Barrier Rail: 0
 241 Bridge Median Height: 0
 * Bridge Median Width: 0
 230 Guardrail Loc. Dir. Rear: 2
 Fwd: 1
 Oppo. Dir. Rear: 0
 Oppo. Fwd: 0
 244 Approach Slab: 3
 224 Retaining Wall: 0
 233Posted Speed Limit: 35
 236 Warning Sign: 0.00
 234 Delineator: 0.00
 235 Hazzard Boards: 0
 237 Utilities Gas: 21
 Water: 22
 Electric: 22
 Telephone: 22
 Sewer: 00
 247 Lighting Street: 0
 Navigation: 0
 Aerial: 0
 *248 County Continuity No.: 00

Processed Date:9/30/2010

Bridge Inventory Data Listing



Parameters: Bridge Serial Num

Structure ID:121-0270-0

Programming Data		Measurements:				
201 Project No:	APD-056-1 (10) CT.2	*29ADT	039350	Year:2007	65 Inventory Rating Method:	1
202 Plans Available:	4	109%Trucks:	0		63 Operating Rating Method:	1
249 Prop Proj No:	000000000000000000000000	* 28 Lanes On:	04	Under:10	66 Inventory Type:	2 Rating: 24
250 Approval Status:	0000	210 No. Tracks On:	00	Under:00	64 Operating Type:	2 Rating: 24
251 PI Number:	0000000	* 48 Max. Span Length	0089		231Calculated Loads:	
252 Contract Date:	02/01/1901	* 49 Structure Length:	289		H-Modified:	21 0
260 Seismic No:	00000	51 Br. Rwdy. Width	52.00		HS-Modified:	28 0
75 Type Work:	00 1	52 Deck Width:	61.00		Type 3:	27 0
94 Bridge Imp. Cost:	\$0	* 47 Tot. Horiz. Cl:	52		Type 3s2:	34 0
95 Roadway Imp. Cost:	0	50 Curb / Sidewalk Width	5.00 / 2.00		Timber:	31 0
96 Total Imp Cost:	0	32 Approach Rdwy. Width	052		Piggyback:	00 0
76 Imp Length:	000000	*229 Shoulder Width:			261 H Inventory Rating:	26
97 Imp Year:	0000	Rear Lt:	2.00	Type:1 Rt:2.00	262 H Operating Rating	43
114Furure ADT:	059025 Year:2027	Fwd. Lt:	2.00	Type:1 Rt:2.00	67 Structural Evaluation:	5
Hydraulic Data		Permanent Width:			58 Deck Condition:	7
215Waterway Data:		Rear:	48.00	Type:1	59 Superstructure Condition:	7
High Water Elev:	0000.0 Year:1900		48.00	Type:2	* 227 Collision Damage:	0
Flood Elev:	0000.0 Freq:00	Intersection Rear:	1	Fwd: 1	60A Substructure Condition:	7
Avg Streambed Elev:	0000.0	36Safety Features Br. Rail:	2		60B Scour Condition:	N
Drainage Area:	00000	Transition:	2		60C Underwater Condition	N
Area of Opening:	000000	App. G. Rail:	1		71 Waterway Adequacy:	N
113 Scour Critical	N	App. Rail End:	1		61 Channel Protection Cond.:	N
216Water Depth:	00.0 Br.Height:00.0	53 Minimum Cl. Over:	99' 99 "		68 Deck Geometry:	4
222Slope Protection:	4	Under:			69 UnderClr. Horz/Vert:	6
221Slope Protection	0 Fwd:0	*228 Minimum Vertical Cl			72 Appr. Alignment:	7
219Fender System	0	Act. Odm Dir.:	99' 99"		62 Culvert:	N
220Dolphin:	0	Oppo. Dir:	99' 99"		Posting Data	
223Current Cover:	000	Posted Odm. Dir:	00' 00"		70 Bridge Posting Required	5
Type:	0	Oppo. Dir:	00' 00"		41 Struct Open, Posted, CL:	A
No. Barrels:	0	55 Lateral Undercl. Rt:	H 12 12		* 103 Temporary Structure:	0
* Width:	0.00 Height:0.00	56 Lateral Undercl. Lt:	4.50		232 Posted Loads	
* Length:	0 Apron:0	*10 Max Min Vert Cl:	99' 99" Dir:0		H-Modified:	00
265 U/W Insp. Area	0 Diver:ZZZ	39 Nav Vert Cl:	000 Horiz:0000		HS-Modified:	00
Location ID No:	121-09260M-005.23N	116 Nav Vert Cl Closed:	000		Type 3:	00
		245 Deck Thickness Main	7.50		Type 3s2:	00
		Deck Thick Approach:	0.00		Timber:	00
		246 Overlay Thickness:	0.00		Piggyback	00
		212 Year Last Painted:	Sup:1994Sub:0000		253 Notification Date:	02/01/1901
					258 Fed Notify Date:	2/1/1901 12:00:00AM